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**IBC2021
COMPLIANT**



ESR-4062



Warranty & Limitations

All products presented herein are warranted to the buyer to be free from defects in material and workmanship. The foregoing warranty is non-assignable and in lieu of and excludes all other warranties not expressly set forth herein, whether express or implied by operation of law or otherwise, including but not limited to any implied warranties of merchantability or fitness for a particular purpose. All details and specifications presented herein are intended as a general guide for the use of MarinoWARE® framing systems. These products should not be used without evaluation by a qualified engineer or architect to determine their suitability for a specific use.

MarinoWARE® assumes no responsibility for failure resulting from use of its details or specifications, or for failure resulting from improper application or installation of these products.

Governing Law

All issues arising in connection with your order and all transactions associated with it shall be interpreted according to the laws of the State of New Jersey, and all actions or other proceedings arising out of such issues shall be brought only in Superior Court, State of New Jersey, County of Essex, or United States District Court for the District of New Jersey. No action may be brought more than one year after accrual of the cause of action therefore.

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ASTM SPECIFICATION DESCRIPTIONS

A1003 - Standard specification for steel sheet, carbon, metallic and nonmetal-coated for cold formed framing members

A653 - Standard specification for steel sheet, zinc-coated (galvanized) or zinc-iron alloy coated by galvanized hot-dip process

A924 - Standard specification for general requirements for steel sheet, metallic-coated galvanized by the hot-dip process

C754 - Standard specification for installation of steel framing members to receive screw-attached gypsum panel products

C955 - Standard specification for load-bearing (transverse and axial) steel studs, runners (tracks), and bracing or bridging for screw application of gypsum panel products and metal plaster bases

CODE COMPLIANCE - ICC-ES ESR#4062

AISI STANDARDS

AISI S100-16/S2-20 North American Specification for the Design of Cold-Formed Steel Structural Members

AISI S240 - North American Standard for Cold-Formed Steel Structural Framing

TECHNICAL SERVICES

Marino\WARE offers its customers free expert technical assistance with the selection and use of our products. If you have questions or need more information on any of the products listed in this catalog, contact our Technical Services department. Our knowledgeable staff is ready to assist you. In most cases Technical Services representatives can provide immediate responses.

Technical Services can be reached at 866-545-1545, or at technicalservices@marinoware.com.

LEED® INFORMATION - MATERIALS & RESOURCES

Marino\WARE® is proud to support the building industry in its efforts to create sustainable commercial and residential buildings. We support the Leadership in Energy & Environmental Design (LEED®) program and have LEED® accredited professionals on staff. Using products manufactured by Marino\WARE® can help in accumulating LEED® points in several categories.

NOMENCLATURE EXAMPLE

600 S 162 - 54

MEMBER DEPTH	TYPE	FLANGE WIDTH	MILS TO GAUGE
250 = 2-1/2"	S = Stud T = Track	137 = 1-3/8"	33 = 20 STR ga.
362 = 3-5/8"		162 = 1-5/8"	43 = 18 ga.
400 = 4"		200 = 2"	54 = 16 ga.
600 = 6"		250 = 2-1/2"	68 = 14 ga.
800 = 8"		300 = 3"	97 = 12 ga.
1000 = 10"		350 = 3-1/2"	118 = 10 ga.
1200 = 12"			
1400 = 14"			
1600 = 16"			

MILS	MINIMUM THICKNESS (in.)	DESIGN THICKNESS (in.)	INSIDE BEND RADIUS (in.)	GAUGE	COLOR CODE	MEMBER	FLANGE WIDTH	RETURN LIP
33	0.0329	0.0346	0.076	20 Structural	WHITE	S137	1-3/8"	0.375
43	0.0428	0.0451	0.071	18	YELLOW	S162	1-5/8"	0.500
54	0.0538	0.0566	0.085	16	GREEN	S200	2"	0.625
68	0.0677	0.0713	0.107	14	ORANGE	S250	2-1/2"	0.625
97	0.0966	0.1017	0.153	12	RED	S300	3"	0.625
118	0.1180	0.1242	0.186	10	BLUE	S350	3-1/2"	1.000

Minimum thickness represents 95% of the design thickness and is the minimum acceptable thickness delivered to the project site based on section B7.1 of AISI S100.



Symbols and Definitions

Gross Properties

I_x	Moment of inertia about x-axis
S_x	Section modulus about x-axis
r_x	Radius about x-axis
I_y	Moment of inertia about y-axis
r_y	Radius about y-axis

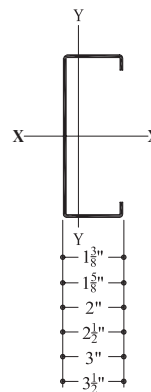
Effective Properties

I_{xd}	Deflection moment of inertia about x-axis
S_{xe}	Section modulus about x-axis
M_{al}	Allowable moment based on local buckling
M_{ad}	Allowable moment based on distortional buckling, assuming $K_b = 0$
M_a	Allowable moment for track and channel sections based on local buckling
V_{ag}	Allowable strong axis shear away from punchout, per AISI Section C3.2.1
V_{anet}	Allowable strong axis shear at punchout, per AISI Section 3.2.2

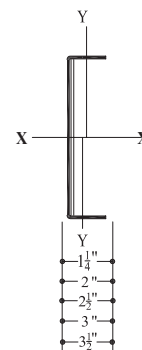
Torsional and other Properties

J	Saint-Venant torsion constant. The values shown in the tables have been multiplied by 1,000. To obtain the actual values, divide table values by 1,000
C_w	Torsional warping constant
x_o	Distance from shear center to centroid along principal x-axis
m	Distance from shear center to mid-plane of web
r_o	Polar radius of gyration about shear center
B	$1 - (x_o / r_o)^2$
L_u	Limit of unbraced length below which lateral-torsional buckling is not considered
K_b	Rotational stiffness

STUD



TRACK



FRAMING MEMBERS

STUD AND JOISTS (CW, SW, J, JE, JX, & JXW)

Studs serve as a general all purpose framing component used in a variety of applications including exterior curtainwalls, load bearing walls, headers, floor & roof joists, soffits and truss frame components.

TRACK (T & DT)

Track is used as a closure to stud and joist ends as well as head and sill conditions. It is also used for blocking and bridging conditions.

C-STUDS (CW, SW, J, JE, JX & JXW)

MIW	GAUGES	WEB	FLANGE	RETURN LIP
CW	20 – 14	2-1/2" – 8"	1-3/8"	3/8"
SW	20 – 10	2-1/2" – 16"	1-5/8"	1/2"
J	20 – 10	2-1/2" – 16"	2"	5/8"
JE•	18 – 10	3-5/8" – 16"	2-1/2"	5/8"
JX•	16 – 10	3-5/8" – 16"	3"	5/8"
JXW•	16 – 10	3-5/8" – 16"	3-1/2"	1"

TRACK (T & DT)

MIW	GAUGES	FLANGE	WEB
T	20 – 10	1-1/4"	2-1/2" – 16"
DT	20 – 10	2" MIN.	2-1/2" – 16"

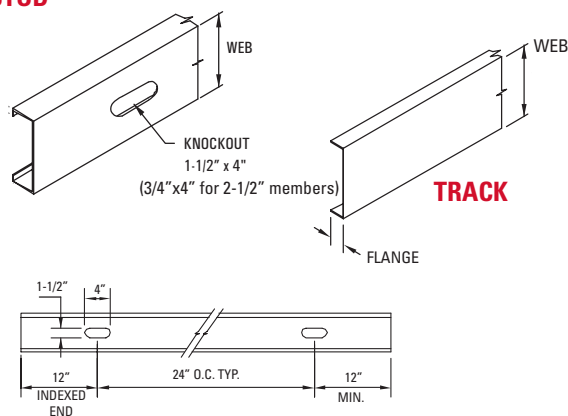
Notes:

1. Products shown with • symbol will be available subject to minimum order quantities.
2. 10'-0" standard length. Custom orders are available.

WEB KNOCKOUT SIZE AND LOCATION

MarinoWARE studs and joists are manufactured with knockouts in the web to accommodate mechanical and electrical installation. The knockout is provided 12" from the indexed end and the intermediate knockouts are placed at 24" o.c. intervals. Unpunched studs are available upon request.

STUD

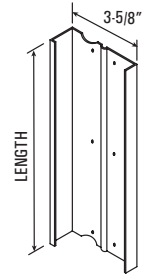


WEB STIFFENERS (JS)

Web Stiffeners are used to provide reinforcement of joist webs to prevent crippling. Web reinforcement is often required by design to enhance the load capacity of joists.

LENGTH: 8", 9 1/4", 10", 11- 1/4, 12", 14"
(inside or outside)

WIDTH: 3 5/8"



INSTALLATION:

- Centered within the load or reaction bearing width.
- Installed on the inside or outside of the joist.
- Web stiffeners require full bearing along their supported ends.
- (4-6) #10 -16 screws are required to attach the stiffener to the joist web using pre-punched holes.

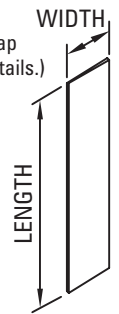
FLAT STRAP

Tension component of shear wall assemblies. Component of strap & blocking for bridging applications. (*See page 25 for more details.)

USE:

Tension component of shear wall assemblies. Component of strap and blocking for bridging application.

AVAILABLE GAUGES: 20, 18, 16, 14, 12 & 10 Gauges.
LENGTH: As required by purchaser.



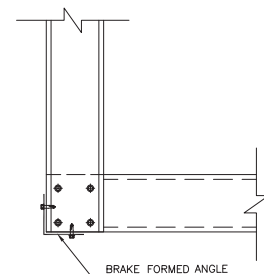
BRAKE FORMED ACCESSORIES

USE:

Miscellaneous closures, continuous angles, etc.

LENGTH:

10'-0" maximum. Dimensioned product drawing must accompany order.

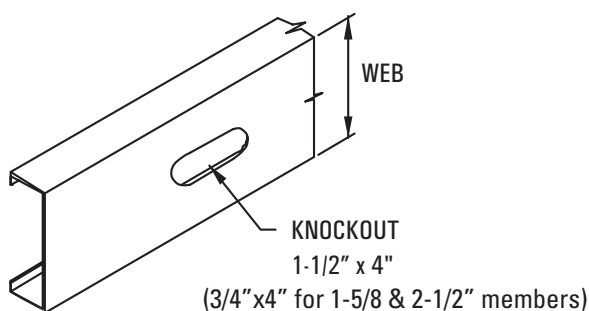


GENERAL NOTES FOR ALL TABLES



1. The information contained in this catalog is intended as a general guide only and all designs shall be verified by a design professional having experience with cold-formed steel design.
2. The data contained in this catalog is based on allowable strength design (ASD) of the 2016 Edition of the North American Specification for the Design of Cold-Formed Steel Structural Members, AISI S100/S2-20).
3. The strength increase based on cold work of forming has been incorporated for flexural strength per Section A3.3.2 of AISI S100.
4. Distortional buckling calculations are based on $K_{\phi} = 0$.
5. The effective moment of inertia for deflection is calculated at a stress that results in a section modulus such that the stress times the section modulus at that stress is equal to the allowable moment. Procedure I of AISI S100 was used for serviceability determination.
6. Various sections may be manufactured with yield stresses of 33 or 50 ksi. The yield stress used for calculations is indicated in the tables.
7. For sections available in both 33 and 50 ksi, the specifier must clearly indicate which yield stress is required.
For example: 362S162-54 (50 ksi).
8. Sections used as nonstructural members that exceed the 10 psf load limit require an approved G60 minimum coating.
9. When provided, factory punchouts shall:
 - be spaced along the centerline of the web of the framing member.
 - have a center-to-center spacing of not less than 24" (610 mm).
 - have a width not greater than half the member depth or 2-1/2" (63.5 mm), whichever is less.
 - have a length not exceeding 4-1/2" (114 mm).
 - The distance from the center of the last punchout to the end of the member shall not be less than 12" (305 mm), unless otherwise specified.

Any configuration or combination of holes that fits within the punchout width and length limitations shall be permitted. Any configuration or combination of holes that fit within the punchout width and length limitations stated above shall be permitted; other punchout configurations and locations not in compliance with the stated limitations must be approved by a design professional.



STRUCTURAL STUD PROPERTIES

Web Depth(h)-to-Thickness (t) Ratios

MIL THICKNESS	18 mil		27 mil		30 mil		33 mil		43 mil		54 mil		68 mil		97 mil		118 mil			
DESIGN THICKNESS (in.)	0.0188		0.0283		0.0312		0.0346		0.0451		0.0566		0.0712		0.1017		0.1242			
INSIDE BEND RADIUS (in.)	0.0844		0.0796		0.0782		0.0765		0.0712		0.0849		0.1069		0.1525		0.1863			
DEPTH (in.)	h (in.)		h/t		h (in.)		h/t		h (in.)		h/t		h (in.)		h/t		h (in.)		h/t	
1.625	1.42	75.5	1.41	49.8	1.41	45.1	1.40	40.5	1.39	30.9	1.34	23.7	1.27	17.8	1.12	11.0	1.00	8.10		
2.5	2.29	122	2.28	80.7	2.28	73.1	2.28	65.8	2.27	50.3	2.22	39.2	2.14	30.1	1.99	19.6	1.88	15.1		
3.625	3.42	182	3.41	121	3.41	109	3.40	98.3	3.39	75.2	3.34	59.0	3.27	45.8	3.12	30.6	3.00	24.2		
4	3.79	202 ¹	3.78	134	3.78	121	3.78	109	3.77	83.5	3.72	65.7	3.64	51.1	3.49	34.3	3.38	27.2		
6	5.79	*	5.78	204 ¹	5.78	185	5.78	167	5.77	128	5.72	101	5.64	79.2	5.49	54.0	5.38	43.3		
8	7.79	*	7.78	*	7.78	249 ¹	7.78	225 ¹	7.77	172	7.72	136	7.64	107	7.49	73.7	7.38	59.4		
10	9.79	*	9.78	*	9.78	*	9.78	*	9.77	217 ¹	9.72	172	9.64	135	9.49	93.3	9.38	75.5		
12	11.8	*	11.8	*	11.8	*	11.8	*	11.8	*	11.7	207 ¹	11.6	163	11.5	113	11.4	91.6		
14	13.8	*	13.8	*	13.8	*	13.8	*	13.8	*	13.7	242 ¹	13.6	191	13.5	133	13.4	108		
16	15.8	*	15.8	*	15.8	*	15.8	*	15.8	*	15.7	219 ¹	15.6	152	15.5	152	15.4	124		

NOTE:

1. h/t exceeds 200.
2. * h/t exceeds 260.

Allowable Screw Connection Capacities (lbs)

GAUGE DESIGNATION	MIL DESIGNATION	DESIGN THICKNESS (in.)	F _y (ksi)	F _u (ksi)	#6 SCREW			#8 SCREW			#10 SCREW			#12 SCREW			#14 SCREW		
					SHEAR	PULL-OUT	PULL-OVER	SHEAR	PULL-OUT	PULL-OVER	SHEAR	PULL-OUT	PULL-OVER	SHEAR	PULL-OUT	PULL-OVER	SHEAR	PULL-OUT	PULL-OVER
25	18	0.0188	33	33*	44	24	84	48	29	84	52	33	105	55	38	105	60	44	127
22	27	0.0283	33	33*	82	37	127	89	43	127	96	50	159	102	57	159	110	66	191
20	30	0.0312	33	33*	95	40	140	103	48	140	111	55	175	118	63	175	127	73	211
20	33	0.0346	33	45	151	61	140	164	72	195	177	84	265	188	95	265	203	110	318
18	43	0.0451	33	45	214	79	140	244	94	195	263	109	345	280	124	345	302	144	415
16	54	0.0566	33	45	214	100	140	344	118	195	370	137	386	394	156	433	424	180	521
14	68	0.0713	33	45	214	125	140	426	149	195	523	173	386	557	196	545	600	227	656
12	97	0.1017	33	45	214	140	140	426	195	195	548	246	386	777	280	775	1016	324	936
16	54	0.0566	50	65	214	140	140	426	171	195	534	198	386	569	225	625	613	261	752
14	68	0.0713	50	65	214	140	140	426	195	195	548	249	386	777	284	775	866	328	948
12	97	0.1017	50	65	214	140	140	426	195	195	548	356	386	777	405	775	1016	468	1067
10	118	0.1242	50	65	214	140	140	426	195	195	548	386	386	777	494	775	1016	572	1067

NOTE:

- * This is a NS type steel with no requirement for ultimate.
- 1. Capacities are based on section J4 of the AISI S100 Specification.
- 2. Capacities are based on Allowable Strength Design (ASD).
- 3. Screw pull-out capacities are based on listed head diameter.
- 4. Two sheets of equal thickness and tensile strength are assumed in tabulated values.
- 5. When materials of different steel thickness and tensile strength are connected, use the lowest value for shear capacity (tilting and bearing), for pull-out capacity use sheet closest to screw tip and for pull-over capacity use sheet closest to screw head.
- 6. Where multiple fasteners are used, screws are assumed to have a center-to-center spacing of at least 3 times the nominal diameter.
- 7. Screws are assumed to have a center-of-screw to edge-of-steel dimension of at least 1.5 times the nominal diameter of the screw.
- 8. When screws are subjected to combination of shear and tension forces, interaction equation of AISI S100 Specification section J4.5 shall be used.
- 9. Screw Shear (P_{ss}), tension (P_{ts}), diameter, and head diameter are from CFSEI Tech Note (F701-12).
- 10. Screw shear strength is the average value, and tension strength is the lowest value listed in CFSEI Tech Note (F701-12).
- 11. Higher values for screw strength (P_{ss}, P_{ts}), may be obtained by specifying screws from a specific manufacturer.

Weld Capacities (lbs/in.)

GAUGE DESIGNATION	MIL DESIGNATION	DESIGN THICKNESS (in.)	F _y (ksi)	F _u (ksi)	FILLET WELDS		FLARE GROOVE WELDS	
					LONGITUDINAL	TRANSVERSE	LONGITUDINAL	TRANSVERSE
18	43	0.0451	33	45	499	864	544	663
16	54	0.0566	33	45	626	1084	682	832
14	68	0.0713	33	45	789	1365	859	1048
12	97	0.1017	33	45	1125	1480	1226	1480
16	54	0.0566	50	65	905	1566	985	1202
14	68	0.0713	50	65	1140	1972	1241	1514
12	97	0.1017	50	65	1480	1480	1480	1480

NOTE:

1. Capacities based on the AISI S100 Specification Sections J2.5 for fillet welds and J2.6 for flare groove welds.
2. When connecting materials of different steel thicknesses or tensile strengths, use the lowest values.
3. Capacities are based on Allowable Strength Design (ASD).
4. Weld capacities are based on E60 electrodes. For material thinner than 68 mil, 0.030" to 0.035" diameter wire electrodes may provide best results.
5. Longitudinal capacity is considered to be loading in the direction of the length of the weld.
6. Transverse capacity is loading in perpendicular direction of the length of the weld.
7. For flare groove welds, the effective throat of weld is conservatively assumed to be less than 2t.
8. For longitudinal fillet welds, a minimum value of Eq. J2.5-1, J2.5-2, J2.5-3, J2.5-4 were used.
9. For transverse fillet welds, a minimum value of Eq. J2.5-5 and J2.5-6 were used.
10. For longitudinal flare groove welds, a minimum value of Eq. J2.6-2 and J2.6-3 were used.
11. For transverse flare groove welds, a minimum value of Eq. J2.6-1 was used.

CURTAINWALL LIMITING HEIGHTS - SINGLE SPAN

NOTES:

- Listed wind pressures represent the calculated design wind pressure (1.0W based on 2009 IBC or 0.6W based on 2012 IBC and later editions). For deflection calculations, the listed wind pressures have been multiplied by 0.70 as per IBC.
- Studs must be braced against rotation and lateral displacement at all supports.
- Studs are assumed to be adequately braced at a maximum spacing of L_u to develop the full allowable moment, M_a .
- Web crippling check is based on 1" of bearing at end supports and 3" of bearing at interior support.
- Shear and web crippling capacity at end supports have NOT been reduced for punchouts. At interior support, the shear and web crippling capacity has been reduced for the presence of punchout adjacent to the supports.
- Combined bending and shear check at interior support is based on unreinforced web as per AISI S100. The shear and combined bending and shear check at interior support have been reduced for the presence of punchouts adjacent to the support.
- Listed "Double Span" limiting heights are based on the distance from wither end to the center of the interior support, with the stud continuous past the interior support.
- "e" - web stiffeners required at ends.
- See General Notes on Page 6.

MEMBER	F _y (ksi)	SPACING (in.) o.c.	15 psf			20 psf			25 psf			30 psf			35 psf			40 psf			50 psf							
			L/240	L/360	L/600	L/240	L/360	L/600	L/240	L/360	L/600	L/240	L/360	L/600	L/240	L/360	L/600	L/240	L/360	L/600	L/240	L/360	L/600					
			362S137-33	33	12	14' 1"	12' 7"	10' 7"	12' 2"	11' 4"	9' 7"	10' 10"	10' 7"	9' 0"	9' 10"	9' 10"	8' 4"	9' 2"	9' 2"	8' 0"	8' 7"	8' 7"	7' 8"	7' 8"	7' 8"	7' 8"	7' 8"	7' 8"

NOTE: See page 18 for Table Notes.

CURTAINWALL LIMITING HEIGHTS - SINGLE SPAN

MEMBER	F _y (ksi)	SPACING (in.) o.c.	15 psf			20 psf			25 psf			30 psf			35 psf			40 psf			50 psf		
			L/240	L/360	L/600	L/240	L/360	L/600	L/240	L/360	L/600	L/240	L/360	L/600	L/240	L/360	L/600	L/240	L/360	L/600	L/240	L/360	L/600
			362S250-43	33	12	18' 3"	16' 0"	13' 6"	16' 7"	14' 6"	12' 2"	15' 4"	13' 6"	11' 4"	14' 0"	12' 8"	10' 8"	13' 0"	12' 0"	10' 2"	12' 2"	11' 6"	9' 8"

NOTE: See page 18 for Table Notes.

CURTAINWALL LIMITING HEIGHTS - SINGLE SPAN

Table with columns: MEMBER, Fy (ksi), SPACING (in.) o.c., and limiting heights for various wind pressures (15 psf to 50 psf) across different member sizes (L/240, L/360, L/600).

NOTE: See page 18 for Table Notes.

CURTAINWALL LIMITING HEIGHTS - DOUBLE SPAN

Table with columns: MEMBER, Fy (ksi), SPACING (in.) o.c., and weight categories (15 psf, 20 psf, 25 psf, 30 psf, 35 psf, 40 psf, 50 psf) with sub-columns for L/240, L/360, and L/600. Each cell contains height values in feet and inches.

NOTE: See page 23 for Table Notes.

CURTAINWALL LIMITING HEIGHTS - DOUBLE SPAN

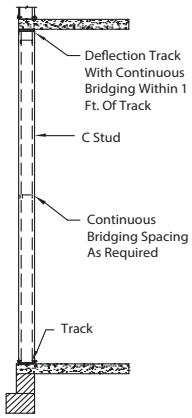
Table with columns: MEMBER, Fy (ksi), SPACING (in.) o.c., and load categories (15 psf, 20 psf, 25 psf, 30 psf, 35 psf, 40 psf, 50 psf) with sub-columns for L/240, L/360, L/600.

NOTE: See page 23 for Table Notes.

CURTAINWALL LIMITING HEIGHTS - DOUBLE SPAN

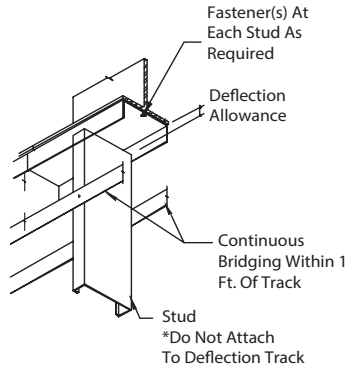
Table with columns: MEMBER, Fy (ksi), SPACING (in.) o.c., and load categories (15 psf, 20 psf, 25 psf, 30 psf, 35 psf, 40 psf, 50 psf). Each load category includes sub-columns for L/240, L/360, and L/600. The table contains multiple rows of height and spacing data for various member types.

NOTE: See page 23 for Table Notes.

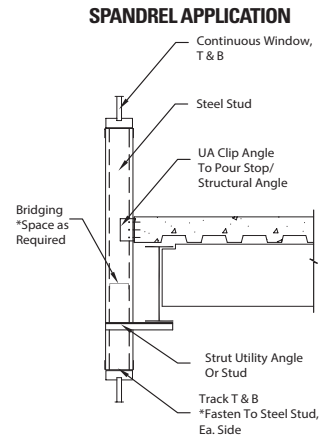


INFILL STUD

INFILL WALL APPLICATION

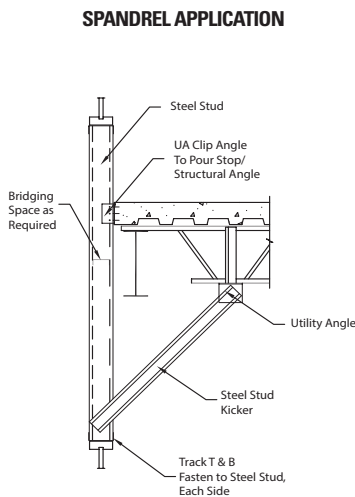


DEFLECTION TRACK



STRUT TO BEAM

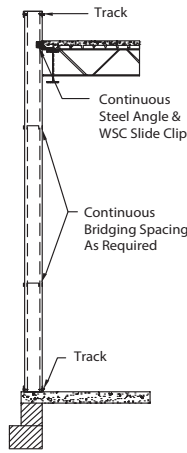
SPANDREL APPLICATION



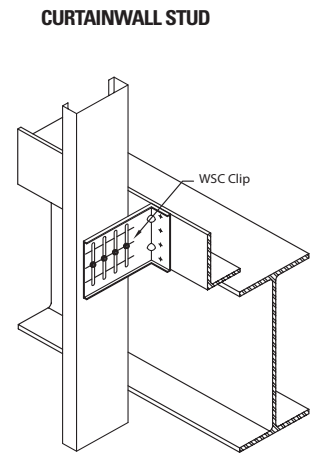
DIAGONAL KICKER

SPANDREL APPLICATION

BY-PASS WALL APPLICATION

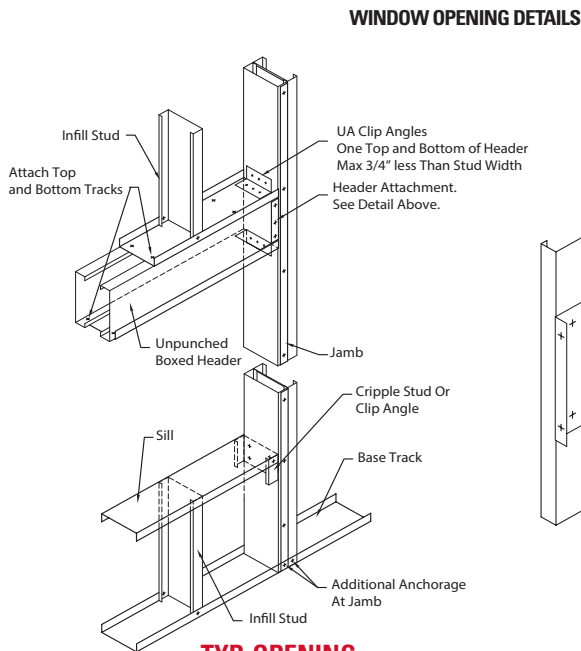


BY-PASS WALL



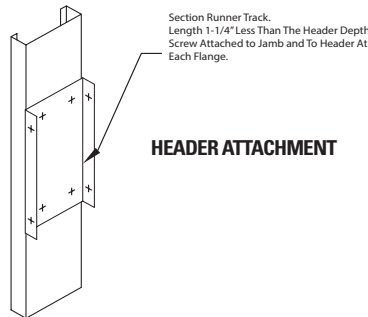
WSC SLIDE CLIP

CURTAINWALL STUD



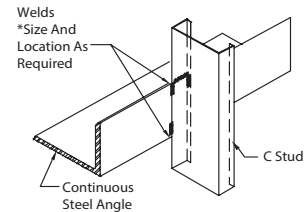
TYP. OPENING

WINDOW OPENING DETAILS

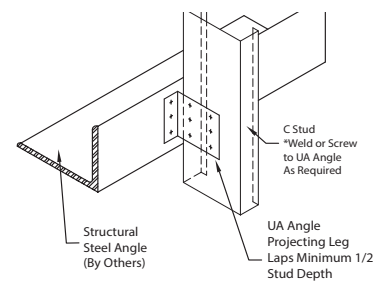


HEADER ATTACHMENT

GRAVITY/LATERAL ATTACHMENT ALTERNATIVES



DIRECT TO STEEL ANGLE



UA ANGLE TO STEEL ANGLE

1. NOTES:
2. Size, spacing and anchorage of framing components shall be qualified by design.
3. Vertical deflection of the primary frame shall be accommodated in the window head.

DIAGONAL CROSS-BRACING

DIAGONAL RACKING BRACING

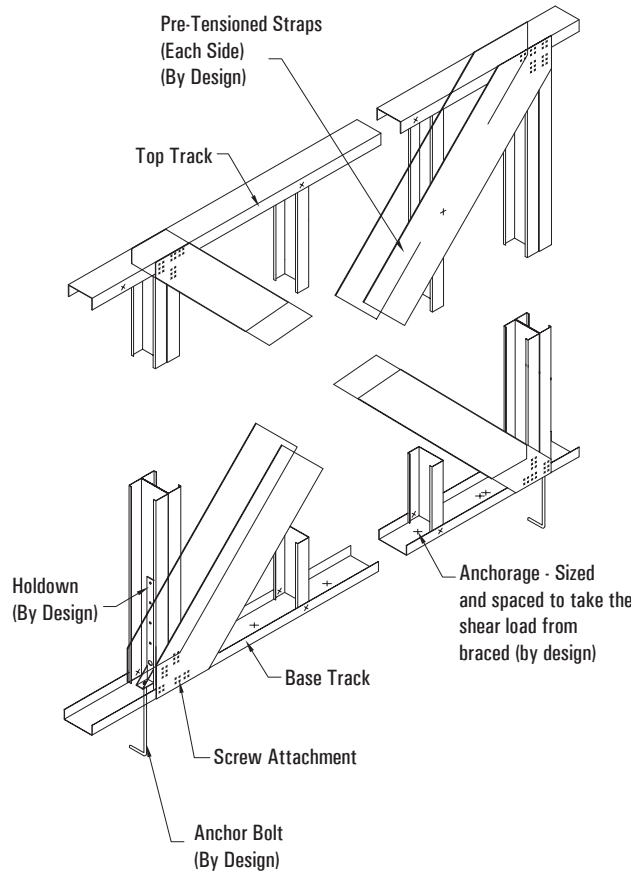
Diagonal bracing and connections must be designed for the specific conditions of a building. For allowable load capacities of Marino\WARE members and assistance in developing connections, please contact Marino\WARE's Technical Services Department. Buildings must be properly braced to resist racking under wind and seismic loads. In steel-framed construction, diagonal strap bracing offers an economical and effective means to provide this resistance. Straps are sloped to resist racking forces in tension. They are installed over framing members and easily covered with facing materials. Diagonal strap ends are secured by screws or welded to transfer the tension loads to the wall framing and floor assembly. End connections must be designed to transfer accumulated design loads. At the foundation, floor anchors must be adequate to prevent uplift and horizontal shear. Splicing of strapping is not recommended.

DIAGONAL CROSS-BRACING

For selection of Flat Strap (FS) subjected to tension loads in shear wall assemblies or miscellaneous applications.

USE:

Select a flat strap which provides an allowable tensile capacity equal to or greater than the applied tension load.



CROSS BRACING: ALLOWABLE TENSION CAPACITY BASED ON SINGLE STRAP		
Flat Strap Bracing (thickness)	Area (in ²)	P _a Without 1/3 increase
2" x 20ga (33mil)	0.0692	1.37
2" x 18ga (43 mil)	0.0902	1.78
4" x 18ga (43 mil)	0.1804	3.57
2" x 16ga(54 mil)	0.1132	3.39
4" x 16ga(54 mil)	0.2264	6.79
2" x 14ga(68 mil)	0.1426	4.28
4" x 14ga(68 mil)	0.2852	8.56

NOTES:

1. Strap end connections shall be designed to transfer the tensile load.
2. 16 and 14 gauge strap: F_y (min) = 50 ksi
3. 20 and 18 gauge strap: F_y (min) = 33 ksi

COMBINED AXIAL AND LATERAL LOADS

15 psf Lateral Load

6" MEMBERS

Table with 16 rows (wall heights 8, 9, 10, 12, 14, 16 ft) and 3 columns (600S137, 600S162, 600S200, 600S250). Each cell contains load values for 33 ksi and 50 ksi at 12, 16, and 24 inch stud spacings.

20 psf Lateral Load

6" MEMBERS

Table with 16 rows (wall heights 8, 9, 10, 12, 14, 16 ft) and 3 columns (600S137, 600S162, 600S200, 600S250). Each cell contains load values for 33 ksi and 50 ksi at 12, 16, and 24 inch stud spacings.

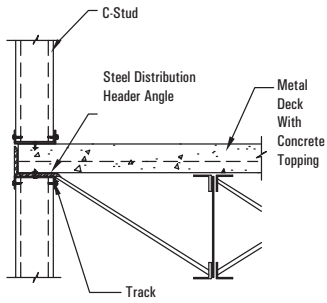
25 psf Lateral Load

6" MEMBERS

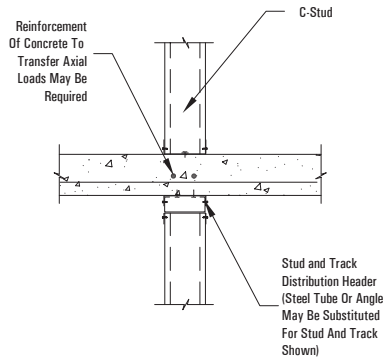
Table with 16 rows (wall heights 8, 9, 10, 12, 14, 16 ft) and 3 columns (600S137, 600S162, 600S200, 600S250). Each cell contains load values for 33 ksi and 50 ksi at 12, 16, and 24 inch stud spacings.

NOTE: See page 30 for Table Notes.

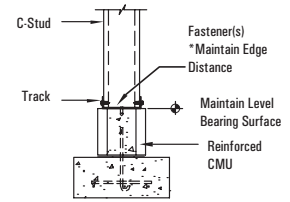
COMBINED AXIAL AND LATERAL LOAD ILLUSTRATIONS



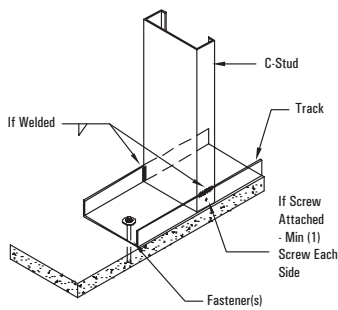
EXTERIOR WALL



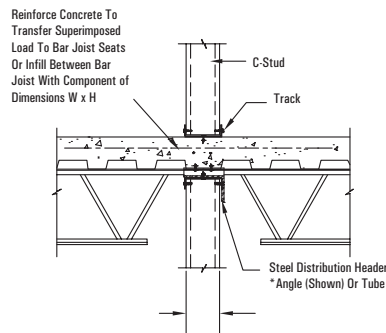
INTERIOR WALL



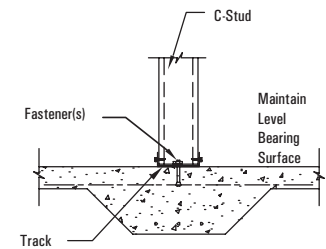
EXTERIOR FOUNDATION



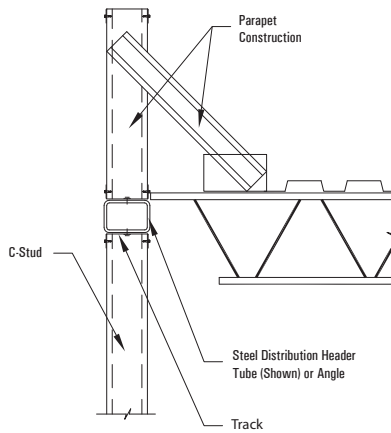
STUD TO TRACK



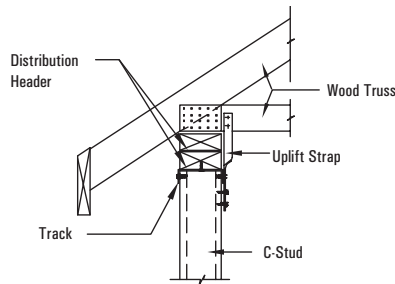
INTERIOR WALL W/ BAR JOIST



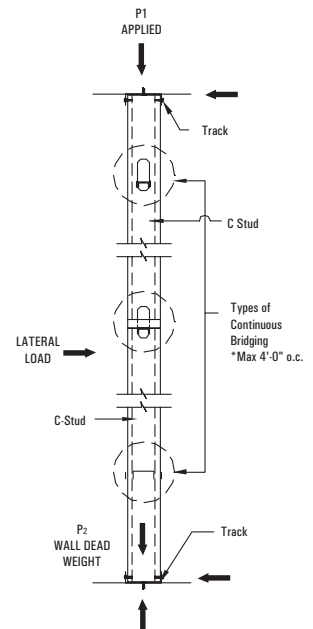
INTERIOR FOOTING



EXTERIOR WALL WITH BAR JOIST



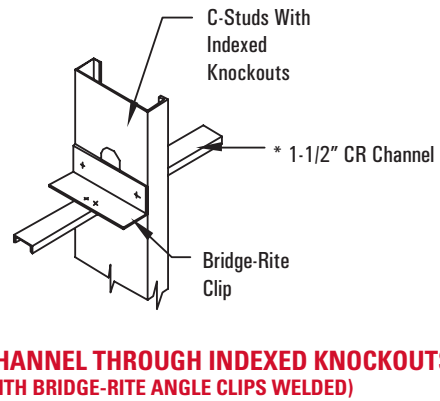
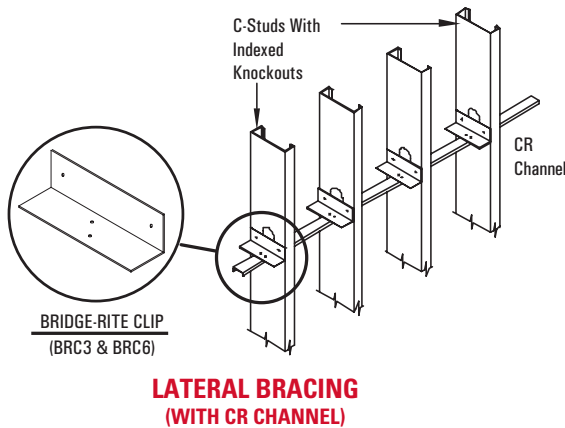
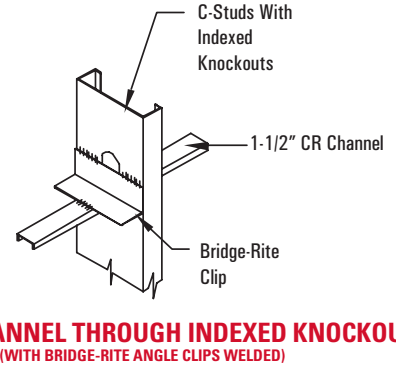
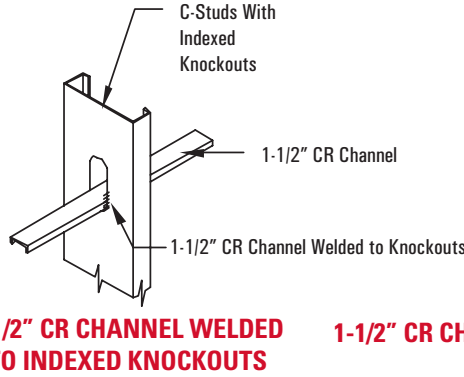
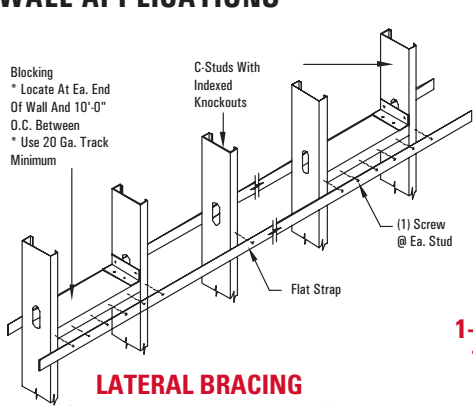
WOOD TRUSS @ EXTERIOR WALL



BEARING WALL SCHEMATIC

P DESIGN = P1 + P2

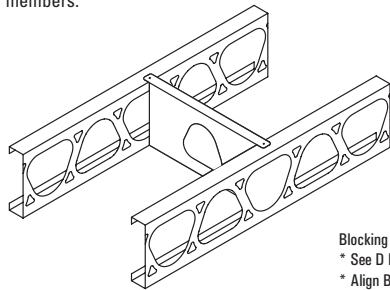
WALL APPLICATIONS



FLOOR & ROOF APPLICATIONS

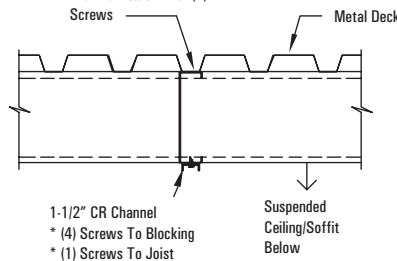
SOLID BLOCKING (JB)

Joist Blocking is pre-cut to fit securely between joists to prevent joist rotation. Joist Blocking is a one piece system in lieu of the typical 3 piece detail offering an economical alternative to installing conventional clips and solid web members.



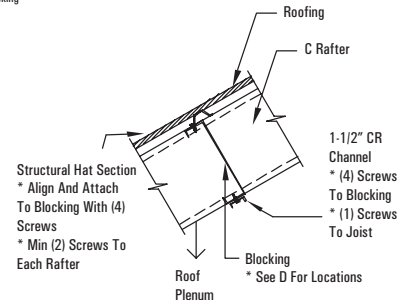
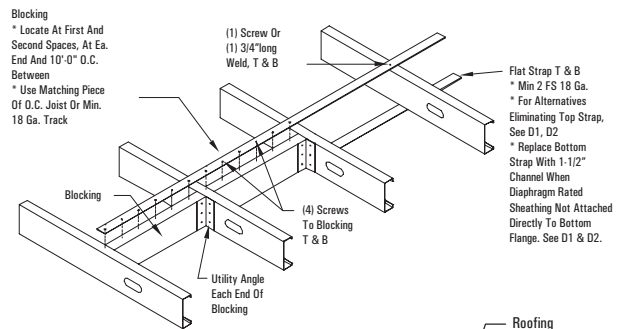
MATERIAL: 16 ga (54 mil) 50ksi
FINISH: Galvanized -G90

Blocking
* See D For Locations
* Align Blocking With Deck Rib And Attach With (4) Screws



FLAT STRAP & BLOCKING (D1 & D2)

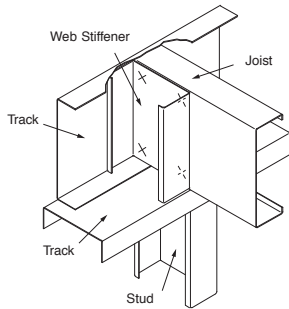
Blocking
• Locate at first and second spaces at ea. end and 10'-0" o.c.
• Use Matching Piece of o.c. Joist or min. 18 Ga. Track



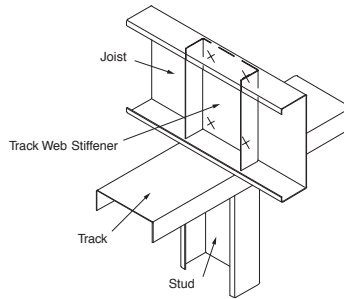
FLOOR JOIST ILLUSTRATIONS

NOTES:

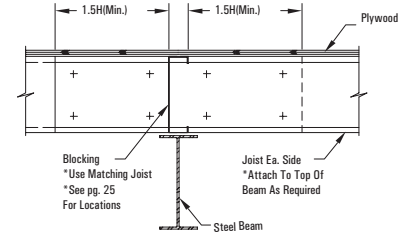
- Spans are based on continuous support of the compression flange over the full length of the joist and the tension flange is laterally braced at maximum spacing of 8'-0".
- For two equal spans, the listed span is the distance from either end to the center support, with the joist continuous over the center support.
- Joists must be braced against rotation at all supports.
- End shear and web crippling capacity have not been reduced for punchouts.
- End web crippling check is based on a 3.5" bearing length.
Where allowable spans are followed by "e", web stiffeners are required at end supports.
- Interior support not checked for combined bending and web crippling. Web stiffeners are required at interior supports.
- Shear capacity at interior support has been reduced for the presence of punchouts adjacent to the supports.
Combined bending and shear check is based on unreinforced web based on AISI S100.
- Total load deflection is limited to L/240. Live load deflection limit is as noted.
- Alternate span live loading has been considered for two equal span conditions.
- Bearing stiffeners must be designed in accordance with AISI S100 section F5, G4.
- See General Notes on Page 6.



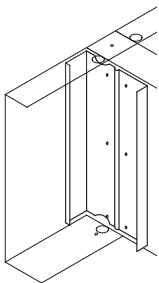
**WEB STIFFENER:
STUD SEGMENT ON BACK OF JOIST**



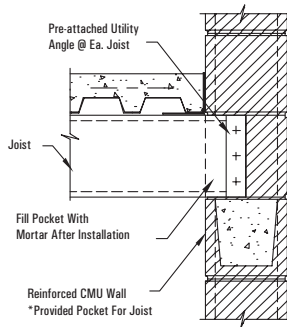
**WEB STIFFENER:
TRACK INSIDE JOIST**



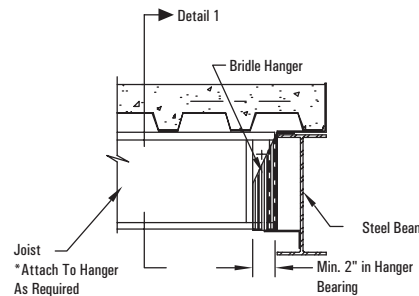
LAPPED JOIST @ INTERIOR SUPPORT



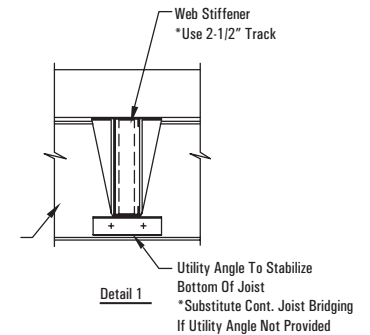
WEB STIFFENER: (JS)



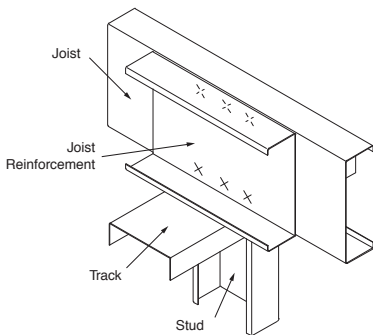
JOIST @ REINFORCED CMU WALL



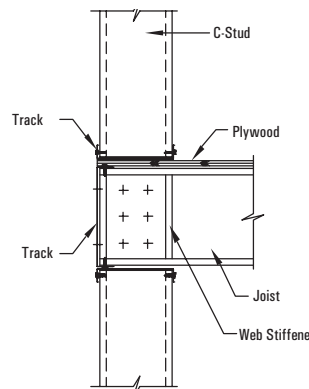
JOIST @ STEEL BEAM W/BRIDLE HANGER



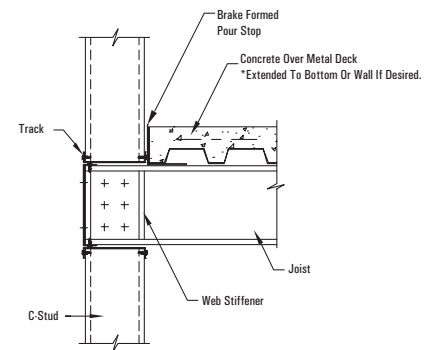
**WEB STIFFENER
*Use 2-1/2" Track**



**WEB STIFFENER: DOUBLE WEB
(USE BACK-TO-BACK WEB CRIPPLING TABLES)**



JOIST @ EXTERIOR BEARING WALL



JOIST @ EXTERIOR BEARING WALL

BRIDGING RECOMMENDATIONS:

Bracing components shall be designed based on AISI S100 (C2) and the minimum number of rows required as shown in table. Additional bridging rows may be required by design. Check fire-rated assemblies for additional requirements.

SPAN (ft)	MINIMUM NUMBER OF ROWS
Up To 16 ft	1 at Mid-span
16 ft To 24 ft	2 Rows at 1/3 Points
24 ft To 32 ft	3 Rows at 1/4 Points

FLOOR JOIST SPANS

10 psf Dead Load and 20 psf Live Load

Table with columns: MEMBER, Fy (ksi), and two main sections for LIVE LOAD DEFLECTION (L/360 and L/480). Each section has sub-columns for SINGLE SPAN and TWO EQUAL SPANS, with further sub-columns for JOIST SPACING (in.) o.c. at 12, 16, and 24 inches.

NOTE: See page 43 for Table Notes.

FLOOR JOIST SPANS

10 psf Dead Load and 50 psf Live Load

Table with columns: MEMBER, Fy (ksi), LIVE LOAD DEFLECTION L/360 (SINGLE SPAN, TWO EQUAL SPANS), LIVE LOAD DEFLECTION L/480 (SINGLE SPAN, TWO EQUAL SPANS). Rows include member numbers like 600S137-33, 800S250-43, etc., and their corresponding joist spacings for various configurations.

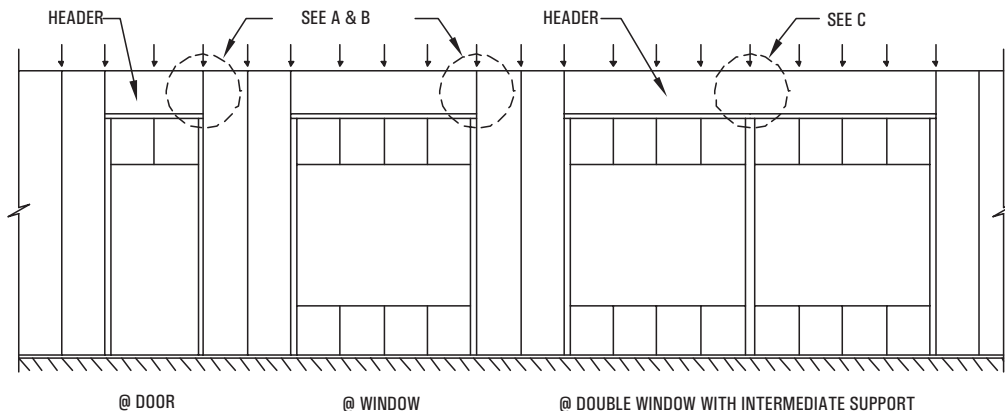
NOTE: See page 43 for Table Notes.

HEADER ALLOWABLE UNIFORM LOADS (lb/ft)

NOTES

- 1. Values are for unpunched members.
2. Total load deflection is limited to L/360.
3. Headers are made from two 'boxed' or back to back members.
4. Allowable moment, shear, web crippling and moment of inertia are based on twice the capacity of a single member.
5. Web crippling check is based on 1" of bearing at end supports.
6. Members are assumed to be adequately braced for bending.
7. Allowable loads are for simply supported headers with uniform bending loads only.
8. 'e' web stiffeners required at ends.
9. See General Notes on Page 6.

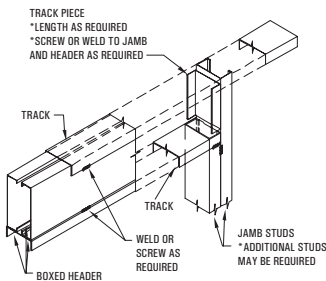
Table with columns: MEMBER, DESIGN THICKNESS (in.), Fy (ksi), and load values for spans 4 through 16 (ft). The table lists allowable uniform loads for various header profiles (e.g., 362S162-33, 600S162-33, etc.) across different design thicknesses and yield strengths.



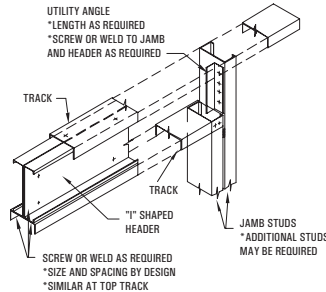
HEADER ALLOWABLE UNIFORM LOADS (lb/ft)

MEMBER	DESIGN THICKNESS (in.)	F _y (ksi)	17	18	19	20	21	22	23	24	25	26	27	28	29	30
			(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)
362S162-33	0.0346	33	-	-	-	-	-	-	-	-	-	-	-	-	-	-
362S162-43	0.0451	33	-	-	-	-	-	-	-	-	-	-	-	-	-	-
362S162-43 (50)	0.0451	50	-	-	-	-	-	-	-	-	-	-	-	-	-	-
362S162-54	0.0566	50	-	-	-	-	-	-	-	-	-	-	-	-	-	-
362S162-68	0.0713	50	-	-	-	-	-	-	-	-	-	-	-	-	-	-
362S162-97	0.1017	50	-	-	-	-	-	-	-	-	-	-	-	-	-	-
362S162-118	0.1242	50	-	-	-	-	-	-	-	-	-	-	-	-	-	-
600S162-33	0.0346	33	-	-	-	-	-	-	-	-	-	-	-	-	-	-
600S162-43	0.0451	33	-	-	-	-	-	-	-	-	-	-	-	-	-	-
600S162-43 (50)	0.0451	50	-	-	-	-	-	-	-	-	-	-	-	-	-	-
600S162-54	0.0566	50	51	-	-	-	-	-	-	-	-	-	-	-	-	-
600S162-68	0.0713	50	63	53	-	-	-	-	-	-	-	-	-	-	-	-
600S162-97	0.1017	50	85	72	61	52	-	-	-	-	-	-	-	-	-	-
600S162-118	0.1242	50	101	85	72	62	53	-	-	-	-	-	-	-	-	-
800S162-33	0.0346	33	60.e	50.e	--	-	-	-	-	-	-	-	-	-	-	-
800S162-43	0.0451	33	80.e	67.e	57.e	-	-	-	-	-	-	-	-	-	-	-
800S162-43 (50)	0.0451	50	78	66	56	-	-	-	-	-	-	-	-	-	-	-
800S162-54	0.0566	50	99	83	71	61	53	-	-	-	-	-	-	-	-	-
800S162-68	0.0713	50	125	106	90	77	67	58	51	-	-	-	-	-	-	-
800S162-97	0.1017	50	173	146	124	106	92	80	70	61	54	-	-	-	-	-
800S162-118	0.1242	50	205	172	147	126	109	94	83	73	64	57	51	-	-	-
1000S162-33	0.0346	33	82.e	73.e	66.e	59.e	52.e	-	-	-	-	-	-	-	-	-
1000S162-43	0.0451	33	119.e	106.e	95.e	82.e	71.e	61.e	54.e	-	-	-	-	-	-	-
1000S162-43 (50)	0.0451	50	129.e	109.e	92.e	79.e	68.e	60.e	52.e	-	-	-	-	-	-	-
1000S162-54	0.0566	50	166.e	140.e	119.e	102	88	76	67	59	52	-	-	-	-	-
1000S162-68	0.0713	50	212.e	179	152	130	112	98	86	75	67	59	53	-	-	-
1000S162-97	0.1017	50	302	254	216	185	160	139	122	107	95	84	75	68	61	55
1000S162-118	0.1242	50	359	302	257	220	190	166	145	128	113	100	90	80	72	65
1200S162-43	0.0451	33	144.e	129.e	116.e	104.e	95.e	86.e	79.e	72.e	63.e	56.e	50.e	-	-	-
1200S162-43 (50)	0.0451	50	163.e	154.e	139.e	120.e	103.e	90.e	79.e	69.e	61.e	54.e	-	-	-	-
1200S162-54	0.0566	50	252.e	212.e	180.e	154.e	133.e	116.e	102.e	89.e	79.e	70.e	63.e	56.e	51.e	-
1200S162-68	0.0713	50	325.e	274.e	233.e	199.e	172.e	150	131	115	102	91	81	73	65	59
1200S162-97	0.1017	50	473.e	399.e	339	291	251	218	191	168	149	132	118	106	95	86
1200S162-118	0.1242	50	572.e	482	410	351	303	264	231	203	180	160	143	128	115	104
1400S162-54	0.0566	50	277.e	261.e	248.e	220.e	190.e	165.e	144.e	127.e	112.e	100.e	89.e	80.e	72.e	65.e
1400S162-68	0.0713	50	433.e	386.e	333.e	285.e	247.e	214.e	188.e	165.e	146.e	130.e	116	104	94	85
1400S162-97	0.1017	50	679.e	579.e	492.e	422.e	364.e	317.e	277.e	244	216	192	171	154	138	125
1400S162-118	0.1242	50	846.e	712.e	606.e	519.e	449.e	390	342	301	266	236	211	189	170	154
1600S162-68	0.0713	50	485.e	446.e	401.e	362.e	328.e	292.e	256.e	225.e	199.e	177.e	158.e	142.e	127.e	115.e
1600S162-97	0.1017	50	793.e	707.e	635.e	573.e	502.e	437.e	382.e	336.e	298.e	265.e	236.e	212	191	172
1600S162-118	0.1242	50	1022.e	912.e	818.e	721.e	623.e	542.e	474.e	417.e	369.e	328	293	263	237	214

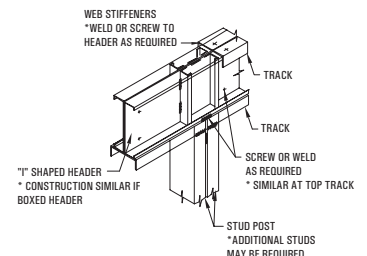
NOTE: See page 56 for Table Notes.



DETAIL A



DETAIL B



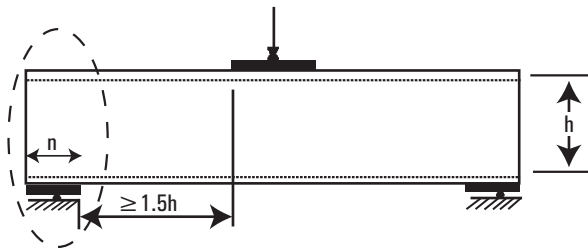
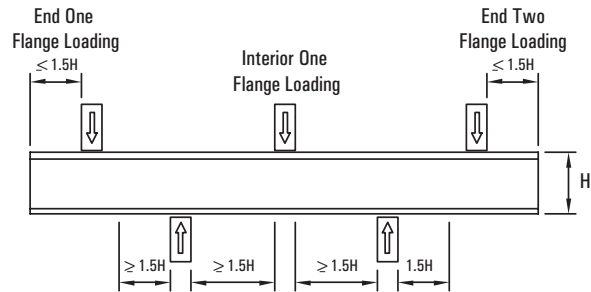
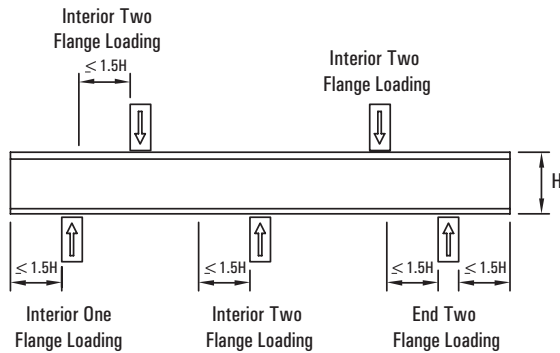
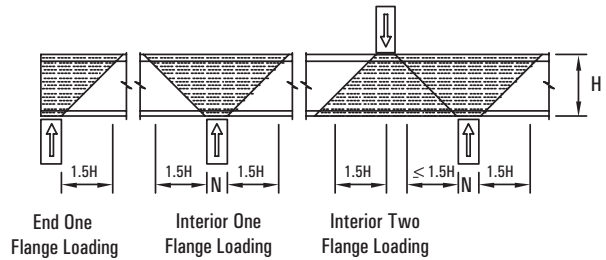
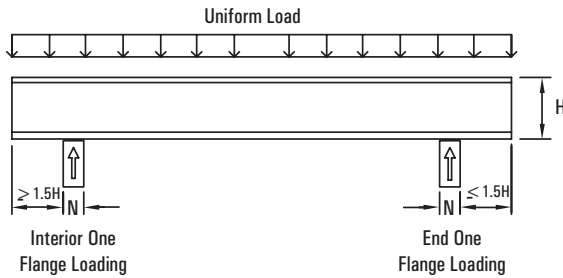
DETAIL C

WEB CRIPPLING LOAD TABLES

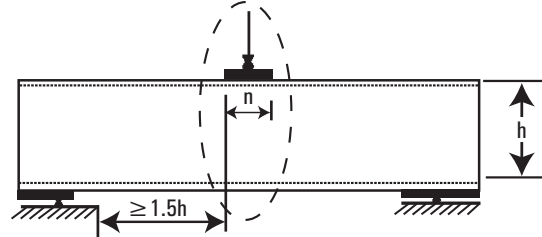
NOTES

1. Listed allowable loads apply only to C-sections.
2. For back-to-back members, the listed allowable loads are for the entire two-member assembly.
3. Listed allowable loads are based on members "fastened to supports", except back-to-back members under two-flange loading (Cases 3 and 4) for which data for "fastened to support" is unavailable in AISI S100.
4. For back-to-back members, the distance between the web connectors and the flange shall be kept to a minimum.
5. Listed allowable loads are for non-punched webs. Capacity reductions for end and interior one flange loading (Cases 1 and 2) near punchouts can be calculated based on AISI S100 (G6).
6. "h" refers to the flat dimension of the web. See web depth-to-thickness ratios table herein.
7. + If $N/h > 2$, then N was set equal to 2h.
8. * If $N/t > 210$, then N was set equal to 210(t)
9. See General Notes on Page 6.

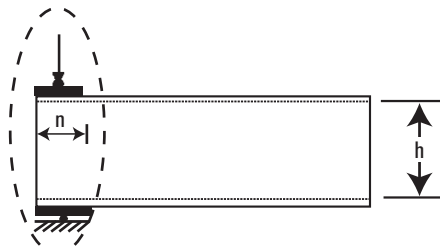
Web Crippling Cases



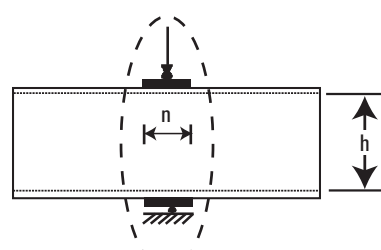
END ONE FLANGE LOADING - CASE 1



INTERIOR ONE FLANGE LOADING - CASE 2



END TWO FLANGE LOADING - CASE 3



INTERIOR TWO FLANGE LOADING - CASE 4

WEB CRIPPLING LOADS

Allowable Web Crippling Loads (lbs) - Back to Back Members

Table with 16 columns: MEMBER, DESIGN THICKNESS (in.), DESIGNATION THICKNESS (mils), Fy (ksi), and cases 1-4 (FASTENED TO SUPPORT BEARING LENGTH) with sub-columns for 1.0, 3.5, and 6.0 in.

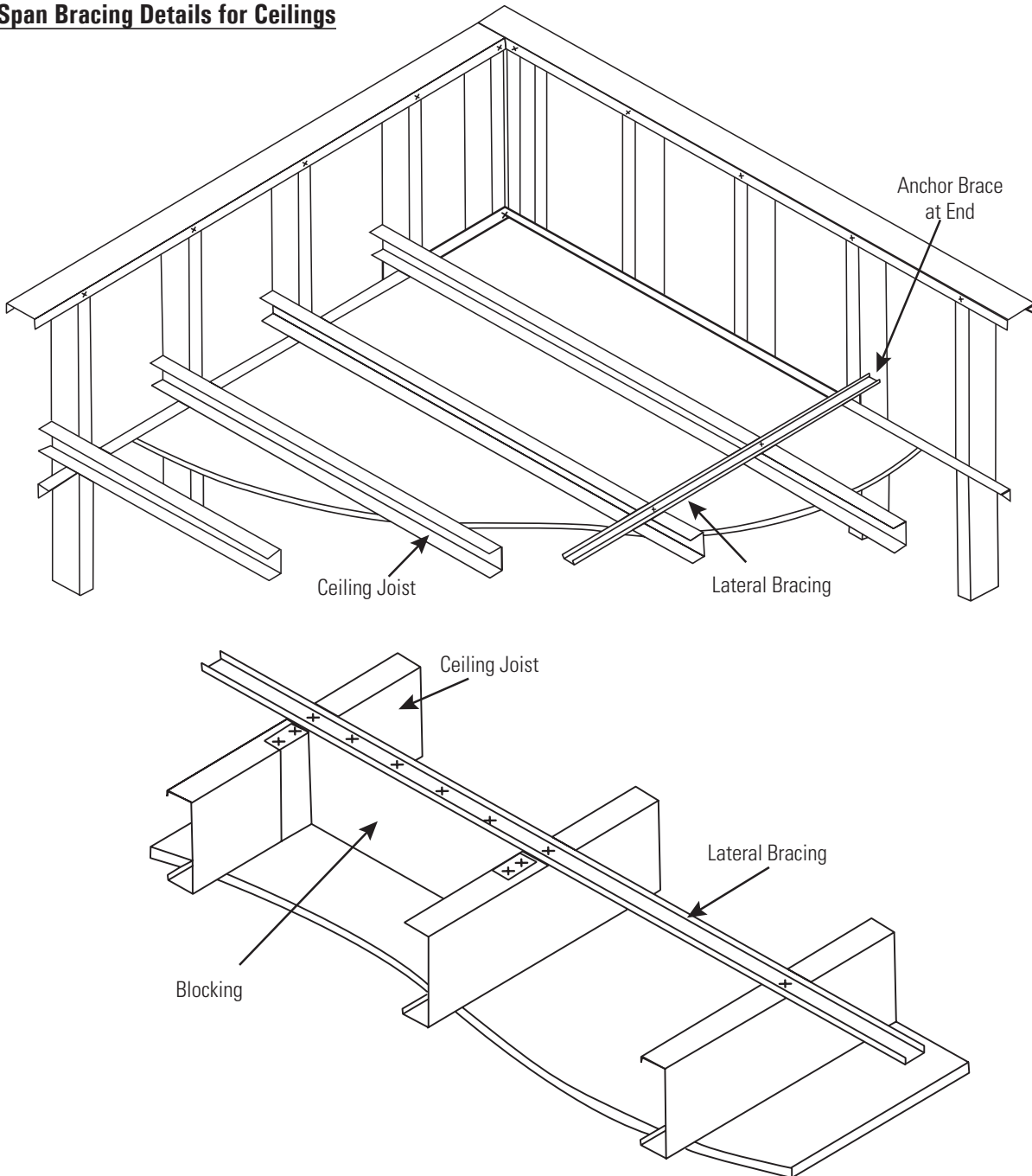
NOTE: See page 58 for Table Notes.

CEILING SPANS (C-SECTIONS)

NOTES

1. Values are for simple span conditions.
2. For unbraced sections, the allowable moment is based on AISI S100 with the unbraced length assumed to be the listed span. For mid-span braced members, the allowable moment is based on AISI S100 with the unbraced length to be half the listed span.
3. Web crippling calculation is based on a bearing length of 1".
4. Web crippling and shear capacity have NOT been reduced for punchouts. If web punchouts occur near supports, members must be checked for reduced shear and web crippling based on AISI S100.
5. "e" indicates that web stiffeners are required at ends.
6. * 13 psf requires a G60 minimum coating.
7. See General Notes on Page 6.

Mid-Span Bracing Details for Ceilings



General Note: All connections should be designed by a licensed design professional.

SECTION PROPERTIES AND CEILING SPANS (U-SECTIONS)

ALLOWABLE CEILING SPAN (U-SECTIONS) NOTES

1. Multiple span indicates two or more equal spans continuous over interior supports.
2. Compression flanges assumed unbraced.
3. Web crippling based on 3/4" bearing at end and interior supports.
4. When applicable, cold work of forming was considered as per Section A3.3.2 of AISI S100-16/S2-20.

Allowable Ceiling Spans (U-Sections) - L/240

MEMBER	F _y (ksi)	SPAN TYPE	4 psf CHANNEL SPACING (in.) o.c.					6 psf CHANNEL SPACING (in.) o.c.					13 psf CHANNEL SPACING (in.) o.c.					15 psf CHANNEL SPACING (in.) o.c.				
			24	36	48	60	72	24	36	48	60	72	24	36	48	60	72	24	36	48	60	72
			75U050-54	33	Single	3' 10"	3' 4"	3' 1"	2' 10"	2' 8"	3' 4"	2' 11"	2' 8"	2' 6"	2' 4"	2' 7"	2' 3"	2' 1"	1' 11"	1' 9"	2' 6"	2' 2"
	33	Multiple	4' 9"	4' 2"	3' 9"	3' 6"	3' 4"	4' 2"	3' 8"	3' 4"	3' 1"	2' 10"	3' 3"	2' 9"	2' 4"	2' 1"	1' 11"	3' 1"	2' 7"	2' 3"	2' 0"	1' 9"
150U050-54	33	Single	5' 6"	4' 10"	4' 5"	4' 1"	3' 10"	4' 10"	4' 3"	3' 10"	3' 7"	3' 4"	3' 9"	3' 3"	3' 0"	2' 9"	2' 7"	3' 7"	3' 2"	2' 10"	2' 8"	2' 6"
	33	Multiple	7' 1"	6' 2"	5' 7"	5' 3"	4' 11"	6' 2"	5' 5"	4' 11"	4' 7"	4' 4"	4' 9"	4' 2"	3' 10"	3' 6"	3' 2"	4' 7"	4' 0"	3' 7"	3' 3"	3' 0"
200U050-54	33	Single	5' 10"	5' 1"	4' 8"	4' 4"	4' 1"	5' 1"	4' 5"	4' 1"	3' 9"	3' 7"	3' 11"	3' 6"	3' 2"	2' 11"	2' 9"	3' 9"	3' 4"	3' 0"	2' 10"	2' 8"
	33	Multiple	7' 5"	6' 6"	5' 11"	5' 6"	5' 2"	6' 6"	5' 8"	5' 2"	4' 10"	4' 6"	5' 0"	4' 5"	4' 0"	3' 9"	3' 6"	4' 10"	4' 3"	3' 10"	3' 7"	3' 4"
250U050-54	33	Single	6' 1"	5' 3"	4' 10"	4' 6"	4' 3"	5' 3"	4' 8"	4' 3"	3' 11"	3' 9"	4' 1"	3' 7"	3' 4"	3' 1"	2' 11"	3' 11"	3' 6"	3' 2"	2' 11"	2' 9"
	33	Multiple	7' 9"	6' 9"	6' 2"	5' 9"	5' 5"	6' 9"	5' 11"	5' 5"	5' 0"	4' 9"	5' 3"	4' 7"	4' 2"	3' 11"	3' 8"	5' 0"	4' 5"	4' 0"	3' 9"	3' 6"

Allowable Ceiling Spans (U-Sections) - L/360

MEMBER	F _y (ksi)	SPAN TYPE	4 psf CHANNEL SPACING (in.) o.c.					6 psf CHANNEL SPACING (in.) o.c.					13 psf CHANNEL SPACING (in.) o.c.					15 psf CHANNEL SPACING (in.) o.c.				
			24	36	48	60	72	24	36	48	60	72	24	36	48	60	72	24	36	48	60	72
			75U050-54	33	Single	3' 4"	2' 11"	2' 8"	2' 6"	2' 4"	2' 11"	2' 7"	2' 4"	2' 2"	2' 0"	2' 3"	2' 0"	1' 9"	1' 8"	1' 7"	2' 2"	1' 10"
	33	Multiple	4' 2"	3' 8"	3' 4"	3' 1"	2' 10"	3' 8"	3' 2"	2' 10"	2' 8"	2' 6"	2' 10"	2' 5"	2' 3"	2' 1"	1' 11"	2' 8"	2' 4"	2' 1"	1' 11"	1' 9"
150U050-54	33	Single	5' 6"	4' 10"	4' 5"	4' 1"	3' 10"	4' 10"	4' 3"	3' 10"	3' 7"	3' 4"	3' 9"	3' 3"	3' 0"	2' 9"	2' 7"	3' 7"	3' 2"	2' 10"	2' 8"	2' 6"
	33	Multiple	7' 1"	6' 2"	5' 7"	5' 3"	4' 11"	6' 2"	5' 5"	4' 11"	4' 7"	4' 4"	4' 9"	4' 2"	3' 10"	3' 6"	3' 2"	4' 7"	4' 0"	3' 7"	3' 3"	3' 0"
200U050-54	33	Single	5' 10"	5' 1"	4' 8"	4' 4"	4' 1"	5' 1"	4' 5"	4' 1"	3' 9"	3' 7"	3' 11"	3' 6"	3' 2"	2' 11"	2' 9"	3' 9"	3' 4"	3' 0"	2' 10"	2' 8"
	33	Multiple	7' 5"	6' 6"	5' 11"	5' 6"	5' 2"	6' 6"	5' 8"	5' 2"	4' 10"	4' 6"	5' 0"	4' 5"	4' 0"	3' 9"	3' 6"	4' 10"	4' 3"	3' 10"	3' 7"	3' 4"
250U050-54	33	Single	6' 1"	5' 3"	4' 10"	4' 6"	4' 3"	5' 3"	4' 8"	4' 3"	3' 11"	3' 9"	4' 1"	3' 7"	3' 4"	3' 1"	2' 11"	3' 11"	3' 6"	3' 2"	2' 11"	2' 9"
	33	Multiple	7' 9"	6' 9"	6' 2"	5' 9"	5' 5"	6' 9"	5' 11"	5' 5"	5' 0"	4' 9"	5' 3"	4' 7"	4' 2"	3' 11"	3' 8"	5' 0"	4' 5"	4' 0"	3' 9"	3' 6"

U-Channel Section Properties

MEMBER	F _y (ksi)	DESIGN THICKNESS (in.)	AREA (in. ²)	WEIGHT (lb/ft)	GROSS PROPERTIES				EFFECTIVE PROPERTIES			
					I _x (in. ⁴)	r _x (in.)	I _y (in. ⁴)	r _y (in.)	I _{xd} (in. ⁴)	S _{xe} (in. ³)	M _a (in.-k)	V _a (lb)
75U050-54	33	0.0566	0.0867	0.295	0.00720	0.288	0.00210	0.156	0.00720	0.0192	0.454	315
150U050-54	33	0.0566	0.129	0.439	0.0387	0.548	0.00271	0.145	0.0387	0.0516	1.22	840
200U050-54	33	0.0566	0.158	0.536	0.0792	0.709	0.00294	0.137	0.0792	0.0792	1.88	1190
250U050-54	33	0.0566	0.186	0.632	0.139	0.866	0.00310	0.129	0.139	0.112	2.64	1540

Note: Inside bend radius taken as 3/32".



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