# Stainless-Steel Titen HD® Heavy-Duty Screw Anchor

Strong-Tie

Cracked

Concrete

The Titen HD stainless-steel screw anchor for concrete and masonry is ideal for when the job calls for fast and efficient installation in multiple types of environments. The Titen HD stainless steel is a state-of-the-art anchor solution that combines the long-lasting corrosion resistance of Type 300 Series stainless steel with the undercutting ability of heat-treated carbon-steel cutting threads.

Innovative — The serrated carbon-steel threads on the tip of the stainless-steel Titen HD are vital because they undercut the concrete as the anchor is driven into the hole, making way for the rest of the threads to interlock with the concrete. In order for these threads to be durable enough to cut into the concrete, they are formed from carbon steel that is then hardened and brazed onto the tip of the anchor.

Corrosion Resistant — For dry, interior applications, carbon-steel corrosion is not a risk, but in any kind of exterior, coastal or chemical environment stainless steel provides the best solution for corrosion protection.

#### Features:

- Ideal for exterior or corrosive environments
- Installs with an impact wrench or with a hand tool
- Tested per ACI355.2, AC193 and AC106

Codes: IAPMO UES ER-493 (concrete);

ICC-ES ESR-1056 (masonry);

City of LA Supplement within ER-493 (concrete);

City of LA Supplement within ESR-1056 (masonry);

Florida FL15730 (masonry); FL16230 (concrete)

Material: Type 316 and Type 304 stainless steel. See pp. 235–236 or visit **strongtie.com/info** for more corrosion information.

#### Installation

- Caution: Holes in steel fixtures to be mounted should match the diameter specified in the table below if steel is thicker than 12 gauge.
- Caution: Use a Titen HD screw anchor one time only installing the anchor multiple times may result in excessive thread wear and reduce load capacity. Do not use impact wrenches to install into hollow CMU.
- Caution: Oversized holes in base material will reduce or eliminate the mechanical interlock of the threads with the base material and reduce the anchor's load capacity.
- 1. Drill a hole in the base material using a carbide drill bit (complying with ANSI B212.15) with the same diameter as the nominal diameter of the anchor to be installed. Drill the hole to the specified minimum hole depth overdrill (see table below) to allow the thread tapping dust to settle, and blow it clean using compressed air. (Overhead installations need not be blown clean.) Alternatively, drill the hole deep enough to accommodate embedment depth and the dust from drilling and tapping.
- 2. Insert the anchor through the fixture and into the hole.
- 3. Tighten the anchor into the base material until the hex-washer head or the countersunk head contacts the fixture.

#### Additional Installation Information

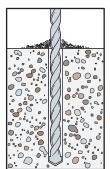
Fiten HD Diameter (in.)	Wrench Size (in.)	Recommended Steel Fixture Hole Size (in.)	Minimum Hole Depth Overdrill (in.)
1/4	3/8	3% to 7/16	1/8
3/8	9/16	½ to %16	1/4
1/2	3/4	5% to 11/16	1/2
5/8	<sup>15</sup> / <sub>16</sub>	3⁄4 to 13⁄16	1/2
3/4	11/8	7⁄8 to 15∕16	1/2

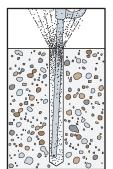
Suggested fixture hole sizes are for structural steel thicker than 12 gauge only. Larger holes are not required for wood or thinner cold-formed steel members.

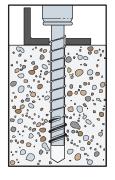


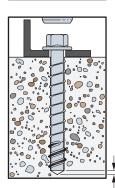
Innovative Carbon-Steel Lead **Threads** 

# Installation Sequence









Stainless-Steel **Titen HD** Hex-Washer **Head Style Screw Anchor** 

> **US Patents** 8,747,042 B2 and 9,517,519

Minimum overdrill See table.

# Stainless-Steel Titen HD® Heavy-Duty Screw Anchor



# Stainless-Steel Countersunk Head Style

The countersunk head style is for applications that require a flush-mount profile. Countersinking also leaves a cleaner surface appearance for exposed through-set applications. The anchor head's 6-lobe drive eases installation and is less prone to stripping than traditional recessed anchor heads.

#### **Features**

- Available in many standard lengths in 1/4" and 3/4" diameters
- Countersunk head allows screw anchor applications incompatible with a hex head
- Countersunk version includes (1) driver bit in each box

Codes: IAPMO UES ER-493 (concrete);

ICC-ES ESR-1056 (masonry);

City of LA Supplement within ER-493 (concrete); City of LA Supplement within ESR-1056 (masonry); Florida FL15730 (masonry); FL16230 (concrete)

Material: Type 316 stainless steel with carbon-steel lead threads

#### Additional Installation Information

Titen HD Diameter (in.)	Bit Size	Recommended Steel Fixture Hole Size (in.)	Minimum Hole Depth Overdrill (in.)
1/4	T30	3/8 to 7/16	1/8
3/8	T50	½ to %16	1/4

Suggested fixture hole sizes are for structural steel thicker than 12 gauge only. Larger holes are not required for wood or thinner cold-formed steel members.

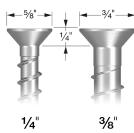
# 6-1/4"



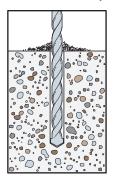


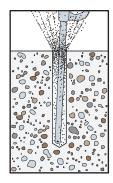


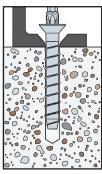
6-lobe drive

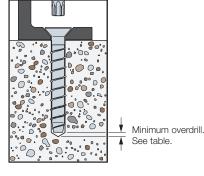


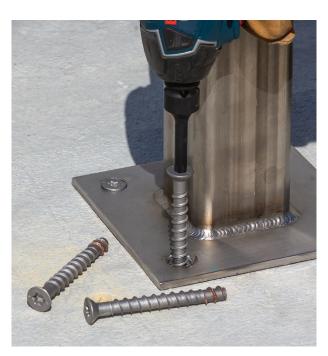
#### Installation Sequence











Titen HD Countersunk Installation

# Stainless-Steel Titen HD® Heavy-Duty Screw Anchor



# Stainless-Steel Titen HD Anchor Product Data — Hex Washer Head

Size	Model No.	Model No.	Thread Length	Drill Bit Diameter	Wrench Size	Quantity		
(in.)	(Type 316)	(Type 304)	(in.)	(in.)	(in.)	Вох	Carton	
1/4 x 2	THDC25200H6SS <sup>†</sup>	_	17/8	1/4	3/8	50	250	
1/4 x 23/8	THDC25238H6SS	_	21/4	1/4	3/8	50	250	
1/4 x 3	THDC25300H6SS	_	27/8	1/4	3/8	50	250	
1/4 x 4	THDC25400H6SS	_	37/8	1/4	3/8	50	250	
3⁄8 x 3	THD37300H6SS	THD37300H4SS	2½	3/8	9/16	50	200	
3/8 X 4	THD37400H6SS	THD37400H4SS	3½	3/8	9/16	50	200	
3⁄8 x 5	THD37500H6SS	THD37500H4SS	41/2	3/8	9/16	50	100	
3⁄8 x 6	THD37600H6SS	THD37600H4SS	5½	3/8	9/16	50	100	
½ x 3	THD50300H6SS <sup>†</sup>	THD50300H4SS <sup>†</sup>	2½	1/2	3/4	25	100	
½ x 4	THD50400H6SS	THD50400H4SS	3½	1/2	3/4	20	80	
½ x 5	THD50500H6SS	THD50500H4SS	41/2	1/2	3/4	20	80	
½ x 6	THD50600H6SS	THD50600H4SS	5½	1/2	3/4	20	80	
½ x 6½	THD50612H6SS	THD50612H4SS	6	1/2	3/4	20	40	
½ x 8	THD50800H6SS	THD50800H4SS	67/8	1/2	3/4	20	40	
5/8 x 4	THDB62400H6SS <sup>†</sup>	THDB62400H4SS†	3½	5/8	15/16	10	40	
% x 5	THDB62500H6SS	THDB62500H4SS	41/2	5/8	15/16	10	40	
% x 6	THDB62600H6SS	THDB62600H4SS	5½	5/8	15/16	10	40	
% x 6½	THDB62612H6SS	THDB62612H4SS	6	5/8	15/16	10	40	
% x 8	THDB62800H6SS	THDB62800H4SS	71/16	5/8	15/16	10	20	
3/4 x 4	THD75400H6SS <sup>†</sup>	THD75400H4SS <sup>†</sup>	3½	3/4	1 1/8	10	40	
3⁄4 x 5	THD75500H6SS <sup>†</sup>	THD75500H4SS <sup>†</sup>	41/2	3/4	1 1/8	5	20	
3⁄4 x 6	THD75600H6SS	THD75600H4SS	5½	3/4	1 1/8	5	20	
3/4 x 7	THD75700H6SS	THD75700H4SS	6½	3/4	1 1/8	5	10	
3/4 X 8 1/2	THD75812H6SS	THD75812H4SS	73/16	3/4	11/8	5	10	

<sup>†</sup> Does not meet minimum embedment in code report.

## Stainless-Steel Titen HD Anchor Product Data — Countersunk

Size	Model No.	Thread	Drill Bit	Bit	Quantity		
(in.)	(Type 316)	Length (in.)	Diameter (in.)	Size	Вох	Carton	
1/4 x 23/8	THDC25238CS6SS <sup>†</sup>	2	1/4	T30	25	250	
1/4 x 3	THDC25300CS6SS	2%	1/4	T30	25	250	
1/4 x 4	THDC25400CS6SS	3%	1/4	T30	25	250	
3/8 X 21/2	THD37212CS6SS <sup>†</sup>	2	3/8	T50	25	125	
3⁄8 x 3	THD37300CS6SS	21/2	3/8	T50	25	125	
3⁄8 X 4	THD37400CS6SS	3½	3/8	T50	25	125	

<sup>†</sup> These models do not meet minimum embedment depth requirements for strength design and require maximum installation torque of 25 ft.-lb. using a torque wrench, driver drill or cordless ¼" impact driver with a maximum permitted torque rating of 100 ft.-lb.

Anchor length is measured from under head to bottom of anchor.

<sup>1.</sup> Anchor length is measured from top of head to bottom of anchor.



#### Stainless-Steel Titen HD Installation Information<sup>1</sup>







Charactaristic	Cumbal	Units				Non	ninal An	chor Di	ameter	(in.)			
Characteristic	Symbol	Units	1	<b>/</b> 4	3,	/8		1/2		5,	/8	3,	/4
Installation Information													
Nominal Diameter	d <sub>a</sub>	in.	1	/4	3	3/8		1/2		5/8		3/4	
Drill Bit Diameter	d <sub>bit</sub>	in.	1	/4	3	<b>½</b>		1/2		5	√8	3,	3/4
Minimum Baseplate Clearance Hole Diameter <sup>2</sup>	$d_{c}$	in.	3	<sup>1</sup> /8	1	/2		5/8		3	3/4	7,	7/8
Maximum Installation Torque <sup>3</sup>	T <sub>inst,max</sub>	ftlbf	N.	/A	4	0		70		8	5	15	50
Maximum Impact Wrench Torque Rating	T <sub>impact,max</sub>	ftlbf	12	25	15	50		345		34	45	38	30
Minimum Hole Depth	h <sub>hole</sub>	in.	21/4	31/8	2¾	3½	3	3/4	41/2	41/2	6	6	6¾
Nominal Embedment Depth	h <sub>nom</sub>	in.	21/8	3	2½	31/4	3	1/4	4	4	5½	5½	61/4
Effective Embedment Depth	h <sub>ef</sub>	in.	1.27	2.01	1.40	2.04	1.	86	2.50	2.31	3.59	3.49	4.13
Critical Edge Distance	Cac	in.	3	3	4½	5½	(	3	5¾	6	6%	6¾	7%
Minimum Edge Distance	C <sub>min</sub>	in.	1½	1½	1¾	13/4	1¾	21/4	13⁄4	13⁄4	1¾	1¾	1¾
Minimum Spacing	S <sub>min</sub>	in.	1½	1½	3	3	4	3	3	3	3	3	3
Minimum Concrete Thickness	h <sub>min</sub>	in.	3½	4%	4	5	ţ	5	61/4	6	8½	8¾	10
		Anchor	Data										
Yield Strength	f <sub>ya</sub>	psi	88,	000	98,	400		91,200		83,	200	92,000	
Tensile Strength	f <sub>uta</sub>	psi	110	,000	123,000 114,000		104	,000	115	,000			
Minimum Tensile and Shear Stress Area	A <sub>se</sub>	in. <sup>2</sup>	0.0	430	0.0	0.099 0.1832		0.276		0.4	114		
Axial Stiffness in Service Load Range — Uncracked Concrete	$eta_{uncr}$	lb./in.	139	,300	807	,700		269,085	5	111,040		102,035	
Axial Stiffness in Service Load Range — Cracked Concrete	$eta_{cr}$	lb./in.	103	,500	113	,540		93,675		94,	400	70,	910

For **SI**: 1 in. = 25.4 mm, 1 ft.-lbf = 1.356 N-m, 1 psi = 6.89 kPa, 1 in.<sup>2</sup> = 645 mm<sup>2</sup>, 1 lb./in. = 0.175 N/mm.

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<sup>1.</sup> The information presented in this table is to be used in conjunction with the design criteria of ACI 318-19 Chapter 17, ACI 318-14 Chapter 17 or ACI 318-11 Appendix D, as applicable.

<sup>2.</sup> The minimum hole size must comply with applicable code requirements for the connected element.

<sup>3.</sup>  $T_{inst,max}$  applies to installations using a calibrated torque wrench.



# Stainless-Steel Titen HD Tension Strength Design Data<sup>1,5</sup>



Characteristic		Units				Nomin	al Ancho	r Diamet	er (in.)			
जावा बहारा हिहार	Symbol	UIIILS	1	/4	3,	/8	1,	/2	5,	/8	3,	/4
Anchor Category	1, 2 or 3	_	;	3				-	1			
Nominal Embedment Depth	h <sub>nom</sub>	in.	21/8	3	2½	31/4	31/4	4	4	5½	5½	61/4
Steel Strength in Tension	n (ACI 318-	19 17.6.	1, ACI 31	8-14 17	.4.1 or A(	CI 318-11	Section	D.5.1)				
Tension Resistance of Steel	N <sub>sa</sub>	lbf	4,7	730	12,	177	20,	885	28,	723	47,	606
Strength Reduction Factor — Steel Failure <sup>2</sup>	$\phi_{sa}$	_					0.	75				
Concrete Breakout Strength in Tension (ACI 318-19 17.6.2, ACI 318-14 17.4.2 or ACI 318 Section D.5.2)												
Effective Embedment Depth	h <sub>ef</sub>	in.	1.27	2.01	1.40	2.04	1.86	2.50	2.31	3.59	3.49	4.13
Critical Edge Distance	Cac	in.	3	3	41/2	5½	6	5¾	6	6%	6¾	73/8
Effectiveness Factor — Uncracked Concrete	K <sub>uncr</sub>	_	24	24	27	24	27	24	24	24	27	27
Effectiveness Factor — Cracked Concrete	K <sub>Cr</sub>	_	17	17	21	17	17	17	17	17	17	21
Modification Factor	$\Psi_{c,N}$	_						1	•			
Strength Reduction Factor — Concrete Breakout Failure <sup>3</sup>	$\phi_{\it cb}$	_	0.	45				0.	65			
Pullout Strength in Tension	on (ACI 318	-19 17.6	5.3, ACI 3	18-14 17	7.4.3 or A	CI 318-1	1 Sectio	n D.5.3)				
Pullout Resistance Uncracked Concrete (f' <sub>c</sub> = 2,500 psi)	N <sub>p,uncr</sub>	lbf	1,7255	3,5508	N/A <sup>4</sup>	N/A <sup>4</sup>	N/A <sup>4</sup>	N/A <sup>4</sup>	3,8205	9,0807	N/A <sup>4</sup>	N/A <sup>4</sup>
Pullout Resistance Cracked Concrete (f' <sub>c</sub> = 2,500 psi)	N <sub>p,cr</sub>	lbf	695⁵	1,225⁵	1,6755	2,415 <sup>5</sup>	1,995 <sup>5</sup>	N/A <sup>4</sup>				
Strength Reduction Factor — Pullout Failure <sup>6</sup>	$\phi_{\mathcal{P}}$	_	0.	45				0.	65			
Tension Strength for Seismic Applic	cations (AC	I 318-19	17.10.3	, ACI 318	3-14 17.2	.3.3 or A	CI 318-1	1 Section	1 D.3.3.3	)		
Nominal Pullout Strength for Seismic Loads (f'c = 2,500 psi)	N <sub>p,eq</sub>	lbf	695⁵	1,2255	1,6755	2,415 <sup>5</sup>	1,9955	N/A <sup>4</sup>				
Strength Reduction Factor for Pullout Failure <sup>6</sup>	$\phi_{eq}$	_	0.	45				0.	65			

For SI: 1 in. = 25.4 mm, 1 ft.-lbf = 1.356 N-m, 1 psi = 6.89 kPa, 1 in.<sup>2</sup> = 645 mm<sup>2</sup>, 1 lb./in. = 0.175 N/mm.

- 1. The information presented in this table is to be used in conjunction with the design criteria of ACI 318-19 Chapter 17, ACI 318-14 Chapter 17 or ACI 318-11 Appendix D, as applicable.
- 2. The tabulated value of φ<sub>sa</sub> applies when the load combinations of Section 1605.1 of the 2021 IBC, Section 1605.2 of the 2018, 2015, 2012, and 2009 IBC, ACI 318-19 and ACI 318-14 Section 5.3, or ACI 318-11 Section 9.2 are used, as applicable. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of φ must be determined in accordance with ACI 318 D.4.4(b), as applicable.
- 3. The tabulated values of  $\phi_{cb}$  applies when both the load combinations of Section 1605.1 of the 2021 IBC, Section 1605.2 of the 2018, 2015, 2012, and 2009 IBC, ACI 318-19 and ACI 318-14 Section 5.3, or ACI 318-11 Section 9.2 are used, as applicable, are used and the requirements of ACI 318-19 Section 17.5.3 and Table 17.5.3(b), ACI 318-14 17.3.3(c) or ACI 318-11 D.4.3(c) for Condition B are met. Condition B applies where supplementary reinforcement is not provided in concrete. For installations where complying reinforcement can be verified, the  $\phi_{cb}$  factors described in ACI 318-19 Table 17.5.3(b), ACI 318-14 17.3.3(c) or ACI 318-11 D.4.3(c), as applicable, may be used for Condition A. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of  $\phi$  must be determined in accordance with ACI 318-11 D.4.4(c) for Condition B.
- 4. N/A denotes that pullout resistance does not govern and does not need to be considered.
- $5. The \ characteristic \ pullout \ resistance \ for \ greater \ compressive \ strengths \ may \ be \ increased \ by \ multiplying \ the \ tabular \ value \ by \ (f'_c/2,500)^{0.5.}$
- 6. The tabulated values of  $\phi_p$  or  $\phi_{eq}$  applies when both the load combinations of ACI 318-19 Section 5.3, ACI 318-14 Section 5.3 or ACI 318-11 Section 9.2, as applicable, are used and the requirements of ACI 318-19 Section 17.5.3 and Table 17.5.3(b), ACI 318-14 17.3.3(c) or ACI 318-11 D.4.3(c) for Condition B are met. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of  $\phi$  must be determined in accordance with ACI 318-11 D.4.4(c) for Condition B.
- 7. The characteristic pullout resistance for greater compressive strengths may be increased by multiplying the tabular value by (f'c/2,500)0.4.
- $8. The characteristic pullout resistance for greater compressive strengths may be increased by multiplying the tabular value by <math>(f'_c/2,500)^{\circ 3}$ .

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# **Stainless-Steel Titen HD®** Design Information — Concrete



# Stainless-Steel Titen HD Shear Strength Design Data<sup>1</sup>

BC	<b>→</b>	[LW]

Characteristic	Symbol	Units	Nominal Anchor Diameter (in.)									
Glidi delelistic	Syllibul	UIIILS	1/4		3/8		1/2		5%		3/4	
Anchor Category	1, 2 or 3	_	3					1				
Nominal Embedment Depth	Nominal Embedment Depth $h_{nom}$ in. 21/8 3						31/4	4	4	5½	5½	61/4
Steel Strength in Shear (ACI 318-19 17.7.1, ACI 318-14 17.5.1 or ACI 318-11 Section D.6.1)												
Shear Resistance of Steel	V <sub>sa</sub>	lbf	2,2	285	3,790	4,780	6,024	7,633	10,422	10,649	13,710	19,161
Strength Reduction Factor — Steel Failure <sup>2</sup>	$\phi_{sa}$	_					0.	65				
Concrete Breakout Strength in Shear (ACI 318-19 17.7.2, ACI 318-14 17.5.2 or ACI 318-11 Section D.6.2)												
Nominal Diameter	da	in.	0.2	250	0.3	375	0.5	500	0.6	625	0.750	
Load Bearing Length of Anchor in Shear	I <sub>e</sub>	in.	1.27	2.01	1.40	2.04	1.86	2.50	2.31	3.59	3.49	4.13
Strength Reduction Factor — Concrete Breakout Failure <sup>3</sup>	$\phi_{\it cb}$	_	0.70						Į.			
Concrete Pryout Strength	in Shear (A	CI 318-1	9 17.7.3	, ACI 318	-14 17.5.	3 or ACI	318-11 S	Section D	.6.3)			
Coefficient for Pryout Strength	K <sub>CP</sub>	_			1.0			2.0	1.0		2.0	
Strength Reduction Factor — Concrete Pryout Failure <sup>3</sup>	$\phi_{cp}$	_					0.	70				
Shear Strength for Seismic App	lications (A	ACI 318-	19 17.10	.3, ACI 31	8-14 17.	2.3.3 or <i>l</i>	ACI 318-1	11 Sectio	n D.3.3.3	)		
Shear Resistance — Single Anchor for Seismic Loads (f' <sub>C</sub> = 2,500 psi)	V <sub>sa,eq</sub>	lbf	1,370	1,600	3,790	4,780	5,345	6,773	9,367	9,367	10,969	10,969
Strength Reduction Factor — Steel Failure <sup>2</sup>	$\phi_{eq}$	_	0.65									

For **SI**: 1 in. = 25.4mm, 1 lbf = 4.45N.

- 1. The information presented in this table is to be used in conjunction with the design criteria of ACI 318-19 Chapter 17, ACI 318-14 Chapter 17 or ACI 318-11 Appendix D, as applicable.
- 2. The tabulated value of  $\phi_{sa}$  and  $\phi_{eq}$  applies when the load combinations of Section 1605.1 of the 2021 IBC, Section 1605.2 of the 2018, 2015, 2012, and 2009 IBC, ACI 318-19 or ACI 318-14 Section 5.3, or ACI 318-11 Section 9.2, as applicable, are used. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of  $\phi_{sa}$  and  $\phi_{eq}$  must be determined in accordance with ACI 318-11 D.4.4(b).
- 3. The tabulated values of  $\phi_{cb}$  and  $\phi_{cp}$  apply when both the load combinations of Section 1605.1 of the 2021 IBC, Section 1605.2 of the 2018, 2015, 2012, and 2009 IBC, ACI 318-19 or ACI 318-14 Section 5.3 or ACI 318-11 Section 9.2 are used and the requirements of ACI 318-19 Section 17.5.3 and Table 17.5.3(b), ACI 318-14 Section 1703.3, or ACI 318-11 D.4.3(c) for Supplementary reinforcement are not present (Condition B) are met. Condition B applies where supplementary reinforcement is not provided in concrete. For installations where complying reinforcement is verified, the  $\phi_{cb}$  and  $\phi_{cp}$  factors described in ACI 318-19 Table 17.5.3(b), ACI 318-14 17.3.3(c), or ACI 318-11 D.4.3(c), as applicable, may be used for Condition A. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of  $\phi_{cb}$  shall be determined in accordance with ACI 318-11 D.4.5(c) for Condition B.



Stainless-Steel Titen HD Screw Anchor Setting Information for Installation on the Top of Concrete-Filled Profile Steel Deck Floor and Roof Assemblies<sup>1,2,3,4</sup>

IBC		<b>→</b>	*
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Design Information	Symbol	Units	Nominal Anchor Diameter (in.)					
Design information	Syllibul	Ullits	1/4	3/8	1/2			
Nominal Embedment Depth	h <sub>nom</sub>	in.	21/8	2½	31/4			
Effective Embedment Depth	h <sub>ef</sub>	in.	1.27	1.40	1.86			
Minimum Concrete Thickness <sup>5</sup>	h <sub>min,deck</sub>	in.	21/2	31⁄4	3¾			
Critical Edge Distance	C <sub>ac,deck,top</sub>	in.	3	4½	7½			
Minimum Edge Distance	C <sub>min,deck,top</sub>	in.	1½	13⁄4	13⁄4			
Minimum Spacing	S <sub>min,deck,top</sub>	in.	1½	3	3			

For SI: 1 in. = 25.4 mm, 1 lbf = 4.45 N.

- 1. For anchors installed in the topside of concrete-filled deck assemblies, as shown in Figure 1, the nominal concrete breakout strength of a single anchor or group of anchors in shear,  $V_{cb}$  or  $V_{cbg}$ , respectively, must be calculated in accordance with ACI 318-19 Section 17.7.2, ACI 318-14 Section 17.5.2 or ACI 318-11 Section D.6.2, using the actual member thickness,  $h_{min,deck}$ , in the determination of  $A_{vc}$ .
- 2. Design capacity shall be based on calculations according to values in the tables featured on pp. 84-85.
- 3. Minimum flute depth (distance from top of flute to bottom of flute) is  $1\frac{1}{2}$ " (see Figure 1).
- 4. Steel deck thickness shall be minimum 20 gauge.
- 5. Minimum concrete thickness (h<sub>min,deck</sub>) refers to concrete thickness above upper flute (see Figure 1).

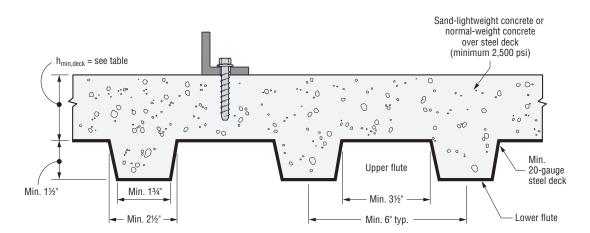


Figure 1. Installation of ¼"-, %"- and ½"-Diameter Anchors in the Topside of Concrete over Steel Deck



Stainless-Steel Titen HD Allowable Tension and Shear Loads in 8" Medium-Weight and Normal-Weight Grout-Filled CMU







Sizo	Size Drill Bit Embedment Pictures				Critical Spacing			Medium-Weight nt Grout-Filled CMU				
in.	Diameter in.		Distance C <sub>crit</sub>	Distance C <sub>min</sub> in. (mm)	Distance in. (mm)	Tensio	n Load	Shear	Load			
(mm)			in. (mm)			Ultimate lb. (kN)	Allowable lb. (kN)	Ultimate lb. (kN)	Allowable lb. (kN)			
Anchor Installed in the Face of the CMU Wall (See Figure 1)												
<b>1/4</b> (6.4)	1/4	<b>2½</b> (64)	<b>4</b> (102)	<b>11/4</b> (32)	<b>4</b> (102)	<b>1,325</b> (5.9)	<b>265</b> (1.2)	<b>1,400</b> (6.2)	<b>280</b> (1.3)			
<b>3/8</b> (9.5)	3/8	<b>2¾</b> (70)	<b>12</b> (305)	<b>4</b> (102)	<b>8</b> (203)	<b>2,125</b> (9.5)	<b>425</b> (1.9)	<b>2,850</b> (12.7)	<b>570</b> (2.5)			
<b>½</b> (12.7)	1/2	<b>3½</b> (89)	<b>12</b> (305)	<b>4</b> (102)	<b>8</b> (203)	<b>3,325</b> (14.8)	<b>665</b> (3.0)	<b>4,950</b> (22.0)	<b>990</b> (4.4)			
<b>5%</b> (15.9)	5/8	<b>4½</b> (114)	<b>12</b> (305)	<b>4</b> (102)	<b>8</b> (203)	<b>3,850</b> (17.1)	<b>770</b> (3.4)	<b>4,925</b> (21.9)	<b>985</b> (4.4)			
<b>3/4</b> (19.1)	3/4	<b>5½</b> (140)	<b>12</b> (305)	<b>4</b> (102)	<b>8</b> (203)	<b>5,200</b> (23.1)	<b>1,040</b> (4.6)	<b>4,450</b> (19.8)	<b>890</b> (4.0)			

- 1. The tabulated allowable loads are based on a safety factor of 5.0 for installations under the IBC and IRC.
- 2. Values for 8"-wide, medium-weight and normal-weight concrete masonry units. For %"- to %"-diameter anchors, anchors may be installed in lightweight masonry units.
- 3. The masonry units must be fully grouted.
- 4. The minimum specified compressive strength of masonry, f'm, at 28 days is 2,000 psi.
- 5. Embedment depth is measured from the outside face of the concrete masonry unit.
- 6. Grout-filled CMU wall design must satisfy applicable design standards and be capable of withstanding applied loads.
- 7. Refer to allowable load-adjustment factors for spacing and edge distance on pp. 89-90.
- 8. Although the ¼" stainless steel Titen HD is not part of the evaluation report, we still tested the ¼" screw per the appropriate AC.

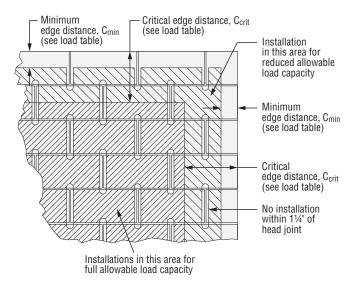


Figure 1. Shaded Area = Placement for Full and Reduced Allowable Load Capacity in Grout-Filled CMU



Stainless-Steel Titen HD Allowable Tension and Shear Loads in 8" Lightweight, Medium-Weight and Normal-Weight Hollow CMU



Cina	Minimur		Critial	Critical	8" Hollow CMU Loads Based on CMU Strength							
Size in. (mm)	Drill Bit Diameter in.	Embedment Depth <sup>4</sup> in. (mm)	Edge Distance in. (mm)	Spacing Distance in.	Tensio	n Load	Shear Load					
(111111)				(mm)	Ultimate lb. (kN)	Allowable lb. (kN)	Ultimate lb. (kN)	Allowable lb. (kN)				
Anchor Installed in Face Shell (See Figure 2)												
3/8 (9.5)	3/8	<b>2½</b> (64)	<b>12</b> (305)	<b>8</b> (203)	<b>925</b> (4.1)	<b>185</b> (0.8)	<b>2,250</b> (10.0)	<b>450</b> (2.0)				
1/2 (12.7)	1/2	<b>2½</b> (64)	<b>12</b> (305)	<b>8</b> (203)	<b>1,025</b> (4.6)	<b>205</b> (0.9)	<b>2,325</b> (10.3)	<b>465</b> (2.1)				
5/8 (15.9)	5/8	<b>2½</b> (64)	<b>12</b> (305)	<b>8</b> (203)	<b>550</b> (2.4)	<b>110</b> (0.5)	<b>2,025</b> (9.0)	<b>405</b> (1.8)				
<b>3/4</b> (19.1)	3/4	<b>2½</b> (64)	<b>12</b> (305)	<b>8</b> (203)	<b>775</b> (3.4)	<b>155</b> (0.7)	<b>1,975</b> (8.8)	<b>395</b> (1.8)				

- 1. The tabulated allowable loads are based on a safety factor of 5.0 for installations under the IBC and IRC.
- 2. Values for 8"-wide, lightweight, medium-weight and normal-weight concrete masonry units.
- 3. The minimum specified compressive strength of masonry, f'm, at 28 days is 2,000 psi.
- 4. Embedment depth is measured from the outside face of the concrete masonry unit and is based on the anchor being embedded an additional 11/4" through 11/4"-thick face shell.
- 5. Allowable loads may not be increased for short-term loading due to wind or seismic forces. CMU wall design must satisfy applicable design standards and be capable of withstanding applied loads.
- 6. Do not use impact wrenches to install in hollow CMU.
- 7. Set drill to rotation-only mode when drilling into hollow CMU.
- 8. Refer to allowable load-adjustment factors for spacing and edge distance on p. 91.
- 9. Anchors must be installed a minimum of 1½" from vertical head joints and T-joints. Refer to Figure 2 for permitted and prohibited anchor installation locations.

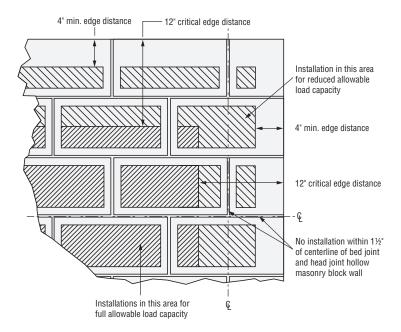


Figure 2. Stainless-Steel Titen HD Screw Anchor Installed in the Face of Hollow CMU Wall Construction



Load-Adjustment Factors for Stainless-Steel Titen HD Anchors in Face-of-Wall Installation in 8" Grout-Filled CMU: Edge Distance and Spacing, Tension and Shear Loads

#### How to use these charts:

- 1. The following tables are for reduced edge distance and spacing.
- Locate the anchor size to be used for either a tension and/or shear load application.
- 3. Locate the embedment (E) at which the anchor is to be installed.
- Locate the edge distance (c<sub>act</sub>) or spacing (s<sub>act</sub>) at which the anchor is to be installed.
- 5. The load adjustment factor ( $f_c$  or  $f_s$ ) is the intersection of the row and column.
- 6. Multiply the allowable load by the applicable load adjustment factor.
- 7. Reduction factors for multiple edges or spacings are multiplied together.

Edge	Distance	Tension	$(f_C)$
------	----------	---------	---------

0		( 0)				
	Dia.	1/4	3/8	1/2	5/8	3/4
	E	21/2	23/4	31/2	41/2	5½
c <sub>act</sub> (in.)	C <sub>cr</sub>	4	12	12	12	12
(111.)	C <sub>min</sub>	1.25	4	4	4	4
	f <sub>cmin</sub>	0.84	0.80	0.81	1.00	1.00
1.25		0.84				
2		0.88				
3		0.94				
4		1.00	0.80	0.81	1.00	1.00
6		1.00	0.85	0.86	1.00	1.00
8		1.00	0.90	0.91	1.00	1.00
10		1.00	0.95	0.95	1.00	1.00
12		1.00	1.00	1.00	1.00	1.00

See footnotes below.

#### Edge Distance Shear (f<sub>c</sub>) Shear Load Parallel to Edge or End

<b>IBC</b>	<b>→</b>		J.
------------	----------	--	----

	Dia.	1/4	3/8	1/2	5/8	3/4
	E	21/2	2¾	31/2	41/2	51/2
c <sub>act</sub> (in.)	C <sub>Cr</sub>	4	12	12	12	12
(111.)	C <sub>min</sub>	1.25	4	4	4	4
	f <sub>cmin</sub>	0.89	0.88	0.56	0.65	0.84
1.25		0.89				
2		0.92				
3		0.96				
4		1.00	0.88	0.56	0.65	0.84
6		1.00	0.91	0.67	0.74	0.88
8		1.00	0.94	0.78	0.83	0.92
10		1.00	0.97	0.89	0.91	0.96
12		1.00	1.00	1.00	1.00	1.00

See footnotes below.

# Edge Distance Shear (f<sub>c</sub>) Shear Load Perpendicular to Edge or End

Shear I	Load	Perpen	dicular	to Ec	dge or	End
(Directe	ed To	wards E	dge o	r End)		

	Dia.	1/4	3/8	1/2	5/8	3/4
	E	21/2	23/4	3 1/2	4 1/2	5 1/2
c <sub>act</sub> (in.)	C <sub>Cr</sub>	4	12	12	12	12
(111.)	C <sub>min</sub>	1.25	4	4	4	4
	f <sub>cmin</sub>	0.33	0.93	0.48	0.66	0.69
1.25		0.33				
2		0.51				
3		0.76				
4		1.00	0.93	0.48	0.66	0.69
6		1.00	0.95	0.61	0.75	0.77
8		1.00	0.97	0.74	0.83	0.85
10		1.00	0.98	0.87	0.92	0.92
12		1.00	1.00	1.00	1.00	1.00

<sup>1.</sup> E = embedment depth (inches).

<sup>2.</sup>  $c_{act}$  = actual end or edge distance at which anchor is installed (inches).

<sup>3.</sup>  $c_{cr}$  = critical end or edge distance for 100% load (inches).

<sup>4.</sup> c<sub>min</sub> = minimum end or edge distance for reduced load (inches).

<sup>5.</sup>  $f_c$  = adjustment factor for allowable load at actual end or edge distance.

 $<sup>6.\,</sup>f_{ccr}$  = adjustment factor for allowable load at critical end or edge distance.  $f_{ccr}$  is always = 1.00.

<sup>7.</sup> f<sub>cmin</sub> = adjustment factor for allowable load at minimum end or edge distance.

<sup>8.</sup>  $f_c = f_{cmin} + [(1 - f_{cmin}) (c_{act} - c_{min}) / (c_{cr} - c_{min})].$ 



Load-Adjustment Factors for Stainless-Steel Titen HD Anchors in Face-of-Wall Installation in 8" Grout-Filled CMU: Edge Distance and Spacing, Tension and Shear Loads (cont.)

#### How to use these charts:

- 1. The following tables are for reduced edge distance and spacing.
- Locate the anchor size to be used for either a tension and/or shear load application.
- 3. Locate the embedment (E) at which the anchor is to be installed.
- Locate the edge distance (c<sub>act</sub>) or spacing (s<sub>act</sub>) at which the anchor is to be installed.
- 5. The load adjustment factor ( $f_c$  or  $f_s$ ) is the intersection of the row and column.
- 6. Multiply the allowable load by the applicable load adjustment factor.
- 7. Reduction factors for multiple edges or spacings are multiplied together.

Edge Distance Shear (f<sub>c</sub>) Shear Load Perpendicular to Edge or End (Directed Away from Edge or End)

IBC T TT

	Dia.	1/4	3/8	1/2	5/8	3/4
	E	21/2	2¾	31/2	4 1/2	51/2
c <sub>act</sub> (in.)	C <sub>cr</sub>	4	12	12	12	12
()	C <sub>min</sub>	1.25	4	4	4	4
	f <sub>cmin</sub>	0.33	0.93	0.48	0.66	0.69
1.25		0.33				
2		0.51				
3		0.76				
4		1.00	0.93	0.48	0.66	0.69
6		1.00	0.95	0.61	0.75	0.77
8		1.00	0.97	0.74	0.83	0.85
10		1.00	0.98	0.87	0.92	0.92
12		1.00	1.00	1.00	1.00	1.00

# Spacing Tension (f<sub>s</sub>)

s<sub>act</sub> (in.)

Dia. E

s<sub>cr</sub> s<sub>min</sub> f<sub>smin</sub>

)					
	1/4	3/8	1/2	5%	3/4
	21/2	2¾	3 1/2	4 1/2	5 1/2
	4	8	8	8	8
	2	4	4	4	4
	0.79	0.81	0.79	0.87	0.78
	0.79				
	0.90				
	1.00	0.81	0.79	0.87	0.78
		0.91	0.90	0.94	0.89
		1.00	1.00	1.00	1.00

# Spacing Shear (f<sub>s</sub>)

. 0	(3)					
	Dia.	1/4	3/8	1/2	5/8	3/4
	E	21/2	23/4	3 1/2	4 1/2	5 1/2
S <sub>act</sub> (in.)	S <sub>cr</sub>	4	6	8	10	12
()	S <sub>min</sub>	2	3	4	5	6
	f <sub>smin</sub>	0.78	1.00	0.86	0.90	0.94
2		0.78				
3		0.89				
4		1.00	1.00	0.86	0.90	0.94
6			1.00	0.93	0.95	0.97
8			1.00	1.00	1.00	1.00

- 1. E = embedment depth (inches).
- $2.s_{act}$  = actual spacing distance at which anchors are installed (inches).
- $3.s_{cr}$  = critical spacing distance for 100% load (inches).
- $4. s_{min} = minimum spacing distance for reduced load (inches).$
- $5.f_s$  = adjustment factor for allowable load at actual spacing distance.
- 6.  $f_{scr}$  = adjustment factor for allowable load at critical spacing distance.  $f_{scr}$  is always = 1.00.
- $7. f_{smin}$  = adjustment factor for allowable load at minimum spacing distance.
- 8.  $f_s = f_{smin} + [(1 f_{smin}) (s_{act} s_{min}) / (s_{cr} s_{min})].$

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# **Stainless-Steel Titen HD®** Design Information — Masonry



Load-Adjustment Factors for Stainless-Steel Titen HD Anchors in Face-of-Wall Installation in 8" Hollow CMU: Edge Distance and Spacing, Tension and Shear Loads

#### How to use these charts:

- 1. The following tables are for reduced edge distance and spacing.
- 2. Locate the anchor size to be used for either a tension and/or shear load application.
- 3. Locate the embedment (E) at which the anchor is to be installed.
- 4. Locate the edge distance ( $c_{act}$ ) or spacing ( $s_{act}$ ) at which the anchor is to be installed.

#### 5. The load adjustment factor (f<sub>c</sub> or f<sub>s</sub>) is the intersection of the row and column.

- 6. Multiply the allowable load by the applicable load adjustment factor.
- 7. Reduction factors for multiple edges or spacings are multiplied together.

#### Edge Distance Tension (f<sub>c</sub>)

			C/		
	Dia.	3/8	1/2	5/8	3/4
	E	21/2	21/2	21/2	21/2
c <sub>act</sub> (in.)	c <sub>cr</sub>	12	12	12	12
	C <sub>min</sub>	4	4	4	4
	f <sub>cmin</sub>	1.00	1.00	1.00	1.00
4		1.00	1.00	1.00	1.00
6		1.00	1.00	1.00	1.00
8		1.00	1.00	1.00	1.00
10		1.00	1.00	1.00	1.00
12		1.00	1.00	1.00	1.00
I.E = embe	edment dep	th (inches).			



- $2. c_{act}$  = actual end or edge distance at which anchor is installed (inches).
- 3.  $c_{cr}$  = critical end or edge distance for 100% load (inches).
- c<sub>min</sub> = minimum end or edge distance for reduced load (inches).
- 5.  $f_C$  = adjustment factor for allowable load at actual end or edge distance.
- 6. f<sub>ccr</sub> = adjustment factor for allowable load at critical end or edge distance.  $f_{ccr}$  is always = 1.00.
- 7. f<sub>cmin</sub> = adjustment factor for allowable load at minimum end or edge distance.
- 8.  $f_c = f_{cmin} + [(1 f_{cmin}) (c_{act} c_{min}) / (c_{cr} c_{min})].$

# Edge Distance Shear (f<sub>c</sub>)

	Dia.	3/8	1/2	5/8	3/4
_	Е	2 1/2	2 1/2	21/2	2 1/2
c <sub>act</sub> (in.)	Ccr	12	12	12	12
(111.)	Cmin	4	4	4	4
	f <sub>cmin</sub>	0.78	0.63	0.55	0.51
4		0.78	0.63	0.55	0.51
6		0.84	0.72	0.66	0.63
8		0.89	0.82	0.78	0.76
10		0.95	0.91	0.89	0.88
12		1.00	1.00	1.00	1.00







# Spacing Tension (f<sub>s</sub>) One Anchor per Cell

	Dia.	3/8	1/2	5/8	3/4
_	E	2 1/2	21/2	2 1/2	21/2
c <sub>act</sub> (in.)	c <sub>cr</sub>	8	8	8	8
(111.)	C <sub>min</sub>	4	4	4	4
	f <sub>cmin</sub>	0.72	0.87	0.89	0.70
4		0.72	0.87	0.89	0.70
6		0.86	0.94	0.95	0.85
8		1.00	1.00	1.00	1.00



See notes below.

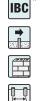
# Spacing Tension (f<sub>s</sub>) Two Anchors per Cell

	Dia.	3/8	1/2	5/8	3/4
	Е	2 1/2	2 1/2	2 1/2	21/2
c <sub>act</sub> (in.)	c <sub>cr</sub>	8	8	8	8
(111.)	C <sub>min</sub>	4	4	4	4
	f <sub>cmin</sub>	1.00	1.00	1.00	0.78
4		1.00	1.00	1.00	0.78
6		1.00	1.00	1.00	0.89
8		1.00	1.00	1.00	1.00



# Spacing Shear (f<sub>s</sub>) One Anchor per Cell

and the second part and						
	Dia.	3/8	1/2	5/8	3/4	
_	E	2 1/2	2 1/2	2 1/2	21/2	
s <sub>act</sub> (in.)	S <sub>cr</sub>	8	8	8	8	
(111.)	Smin	4	4	4	4	
	f <sub>smin</sub>	0.81	1.00	0.71	0.74	
4		0.81	1.00	0.71	0.74	
6		0.91	1.00	0.86	0.87	
8		1.00	1.00	1.00	1.00	



- 1. E = embedment depth (inches).
- 2. s<sub>act</sub> = actual spacing distance at which anchors are installed (inches).
- 3.  $s_{cr}$  = critical spacing distance for 100% load (inches).
- 4. s<sub>min</sub> = minimum spacing distance for reduced load (inches).
- 5.  $f_s$  = adjustment factor for allowable load at actual spacing distance.
- $6.\,f_{scr}$  = adjustment factor for allowable load at critical spacing distance.  $f_{scr}$  is always = 1.00.
- 7. f<sub>smin</sub> = adjustment factor for allowable load at minimum spacing distance.
- 8.  $f_s = f_{smin} + [(1 f_{smin}) (s_{act} s_{min}) / (s_{cr} s_{min})].$

	Dia.	3/8	1/2	5/8	3/4	
_	E	2 1/2	21/2	2 1/2	2 1/2	
c <sub>act</sub> (in.)	c <sub>cr</sub>	8	8	8	8	
(111.)	C <sub>min</sub>	4	4	4	4	
	f <sub>cmin</sub>	1.00	1.00	1.00	0.78	]
4		1.00	1.00	1.00	0.78	
6		1.00	1.00	1.00	0.89	
8		1.00	1.00	1.00	1.00	]

# Spacing Shear (f<sub>s</sub>) Two Anchors per Cell

TWO ALICHOIS PER CEII						
	S <sub>act</sub> (in.)	Dia.	3/8	1/2	5/8	3/4
		E	21/2	21/2	21/2	21/2
		Scr	8	8	8	8
		Smin	4	4	4	4
		f <sub>smin</sub>	0.76	1.00	0.75	0.75
	4		0.76	1.00	0.75	0.75
	6		0.88	1.00	0.88	0.88
	8		1.00	1.00	1.00	1.00



